

Memo: Anderson Russell – Traffic Impact Analysis Addendum

Date:	09/11/17
TO:	City of Maricopa
FROM:	Eric Maceyko, P.E., PTOE
	Bryan A. Martin, P.E.

INTRODUCTION

This traffic analysis is being conducted for the proposed Anderson Russell mixed-use development generally located at Anderson Road and south of the Maricopa-Casa Grande Highway in the City of Maricopa. The traffic analysis will be revised to update the previous *Anderson Russell Traffic Impact Analysis*, prepared in August 2014 by Trace Consulting. It also incorporates revisions based comments received from the City of Maricopa on 12 February 2015, 11 August 2015, 2 August 2017, and 29 August 2017. A copy of the comments is included in **Attachment A**.

This revised traffic analysis is being conducted to prepare an addendum to the previously submitted traffic impact analysis to account for the changes in the proposed land use plan and current long range transportation plans.

BACKGROUND

Figure 1 provides a vicinity map of the study location. The proposed site contains agricultural uses. The land in the immediate vicinity contains mostly agricultural and very low density residential uses, and vacant land. There are also some industrial uses and a small airport immediately to the north.

The previously submitted development land use plan contained a mixture of uses including:

- 2,080 single-family dwelling units
- 291,000 square feet of retail uses
- 872,000 square feet of office uses

Pertinent excerpts from the previous report are contained in Attachment B.

The current land use plan for the proposed mixed-use development expresses some uses by acreage. For business park and commercial retail uses, a floor area ratio of 0.25 was applied to convert the acreage to square footage. The following land uses are included:

- 1,423 single-family dwelling units
- 473 medium-density dwelling units
- 458 high-density dwelling units
- 312,434 square feet of retail uses
- 1,135,500 square feet of business park uses
- 12.15 acre elementary school (assumed 600 student enrollment)

Figure 2 provides a site plan for the current proposed development land use plan.

The current plan is anticipated to be constructed in three (3) phases. **Figure 3** provides the current phasing plan. For the purposes of this study, Phase 1 is assumed to open by 2020, Phase 2 is assumed to open by 2025, and Phase 3 is assumed to open by 2030.



Figure 1: Vicinity Map



Figure 2: Proposed Development Site Plan



Figure 3: Phasing Plan

Anderson Road is currently a two-lane paved roadway with a posted speed limit of 50 mph. It provides an approximately 7 mile continuous north-south route from Maricopa-Casa Grande Highway to State Route 84. There is also currently an offset alignment north of Maricopa-Casa Grande Highway approximately 110 west of the south alignment. This roadway will provide primary direct access to the Anderson Russell mixed-use development. It is planned to be the sole direct access roadway until additional east-west roadway connections are constructed.

Maricopa-Casa Grande Highway is currently a two-lane paved roadway with a posted speed limit of 50 mph. Maricopa-Casa Grande Highway will be the primary regional access road for the proposed development with access to central Maricopa to the west and Casa Grande to the east.

State Route 84 is currently a two-lane paved roadway with a posted speed limit of 55 mph. This roadway will provide alternate east-west regional access, as no other continuous east-west routes currently intersect Anderson Road.

As shown previously in **Figure 2**, the proposed development will be served by a system of collector roadways that intersect Anderson Road south of Maricopa-Casa Grande Highway. For the purposes of this report, these roadways have been labeled as Access 1, Access 2, and Access 3 to remain consistent with the previous report.

The *City of Maricopa Area Transportation Plan*, prepared in December 2015 by Wilson & Company, contains the most current long range transportation plans for the surrounding region. Future plans indicate that the north Anderson Road alignment will be reconstructed to match the south Anderson Road alignment to create a four-way intersection at Maricopa-Casa Grande Highway. This realignment is anticipated to occur by the year 2020. In addition to planned upgrades to both Anderson Road and Maricopa-Casa Grande Highway, additional east-west through roadways are planned. The final timing and alignment for these roadways have not been defined but are planned to exist near the southern end of the proposed development.

EXISTING & FUTURE TRAFFIC VOLUMES

Historical traffic data is available on Maricopa-Casa Grande Highway just west of Anderson Road (provided by MAG) and on Anderson Road south of Maricopa-Casa Grande Highway (provided by Maricopa). The following data is provided:

Maricopa-Casa Grande Highway

- 5,250 vehicles per day (2011)
- 7,059 vehicles per day (2015)
- 7,080 vehicles per day (2016)

Anderson Road

• 922 vehicles per day (2013)

Additionally, morning and evening peak hour turning movement volumes were collected in 2014 at the Maricopa-Casa Grande Highway and Anderson Road intersection. These volumes were reported in the previous Trace Consulting study. As not much development has occurred on Anderson Road, and almost no growth was observed on Maricopa-Casa Grande Highway for the most recent years, the previous turning movement volumes were assumed still valid. **Figure 5** provides the existing 2017 morning and evening peak hour turning movement volumes.

Evaluation of the historic traffic count data on Maricopa-Casa Grande Highway indicates an average annual growth rate of approximately 6% per year for the available time period. The previous *Anderson Russell Traffic Impact Analysis* utilized a projected growth rate of 3.71% per year. Many future developments and new roadway corridors are planned per the *Area Transportation Plan*. This document also provides estimated future traffic volumes for various years based on the anticipated buildout of the region. Therefore, the traffic volumes from this report were utilized as the most reliable estimate of future traffic volumes. It is unclear if the future traffic volumes already account for some or all of the proposed Anderson Russell mixed-use development. Therefore, to provide a conservative analysis, it was assumed that none of the proposed development traffic volumes were already accounted for.

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Figure 4: Existing 2017 Traffic Volumes – AM & PM Peak Hours

The *Area Transportation Plan* provides future traffic volumes on area roadways for the years 2020, 2030, and 2040. The volumes for each of these years include an assumed amount of growth in the known adjacent planned developments as of the time of the report. **Table 1** provides the road segments adjacent to the proposed development with their respective existing and future anticipated daily traffic volumes.

Roadway	From	То	Existing (2015)	2020	2030	2040
Anderson Road	Peters and Nall	MCGH	700	800	1,600	5,500
Anderson Road	MCGH	Miller	1,200	2,100	2,600	6,300
MCGH	Murphy	Anderson	7,400	11,000	17,800	24,800
MCGH	Anderson	Anderson	7,100	10,800	17,800	23,700
MCGH	Anderson	Russell	6,500	9,000	15,500	18,600

Table 1: Study Area Road Segment Existing and Future Daily Traffic

Table 2 provides the road segments adjacent to the proposed development with their calculated growth rate.

Roadway	From	То	2015 to 2020	2020 to 2030	2030 to 2040
Anderson Road	Peters and Nall	MCGH	2.7%	7.2%	13.2%
Anderson Road	MCGH	Miller	11.8%	2.2%	9.2%
MCGH	Murphy	Anderson	8.3%	4.9%	3.4%
MCGH	Anderson	Anderson	8.8%	5.1%	2.9%
MCGH	Anderson	Russell	6.7%	5.6%	1.8%

Table 2: Study Area Road Segment Calculated Growth Rates

The above growth rates were applied linearly to the respective existing traffic volumes to obtain estimated future ambient traffic volumes. **Figure 5** through **Figure 7** provides the future anticipated ambient 2020, 2025, and 2030 morning and evening peak hour turning movement volumes, respectively. Year 2025 traffic volumes were estimated by interpolation between the estimated year 2020 and 2030 traffic volumes.

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Figure 5: Ambient 2020 Traffic Volumes – AM & PM Peak Hours

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Figure 6: Ambient 2025 Traffic Volumes – AM & PM Peak Hours

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Figure 7: Ambient 2030 Traffic Volumes – AM & PM Peak Hours

TRIP GENERATION

The estimated trip generation for the proposed development was determined through the procedures and data contained within the Institute of Transportation Engineers (ITE) *Trip Generation*, 9th Edition, published in 2012. This document provides traffic volume data from existing developments throughout North America that can be utilized to estimate vehicle trips that might be generated from proposed developments. The traffic data are provided for 172 different categories. The estimated traffic volume is dependent upon independent variables defined by the characteristics and size of each land use category.

ITE Land Use Code 210 – Single-Family Detached Housing, was utilized for the single-family residential parcels. The most easily determined independent variable for a typical single-family residential project is the number of dwelling units. Both equations and average rates are provided in *Trip Generation*. Both methods were calculated separately for each time period. The largest volumes considering both calculation methods were utilized as the estimate for the generated traffic for the single-family residential parcels.

ITE Land Use Code 230 – Residential Condominium/Townhouse, was utilized for the medium-density residential parcels. The most easily determined independent variable for a typical medium-density residential project is the number of dwelling units. Both equations and average rates are provided in *Trip Generation*. Both methods were calculated separately for each time period. The largest volumes considering both calculation methods were utilized as the estimate for the generated traffic for the medium-density residential parcels.

ITE Land Use Code 220 – Apartment, was utilized for the high-density residential parcels. The most easily determined independent variable for a typical multi-family residential project is the number of dwelling units. Both equations and average rates are provided in *Trip Generation*. Both methods were calculated separately for each time period. The largest volumes considering both calculation methods were utilized as the estimate for the generated traffic for the multi-family parcels.

The exact nature and size of the business park and commercial parcels are currently unknown. ITE Land Use Code 770 – Business Park and ITE Land Use Code 820 – Shopping Center, were utilized for this study. The most easily determined independent variable for these types of projects is 1,000 Square Feet Gross Floor Area. Volumes utilizing this independent variable were calculated for each time period. Also both equations and average rates are provided in *Trip Generation*. Both methods were calculated separately for each

time period. The largest volumes considering both calculation methods were utilized as the estimate for the generated traffic for the business park and commercial parcels.

The school is anticipated to be kindergarten through 6th grade. ITE Land Use Code 520 – Elementary School, was utilized for this study. The most reliable and easily determined independent variable available for this land use category to predict trips is the number of students. It was assumed for the purposes of this analysis that a total of 600 students can be accommodated. Volumes utilizing this independent variable were calculated for each time period. It is recognized that the majority of potential trips by students attending the school site will be accomplished by use of the internal roadways alone and will not impact the adjacent arterial road network. Therefore, the anticipated trip generation for the school site was reduced by 85% to account for the internal trips.

Table 3 summarizes the total Phase 1 trip generation for the proposed development during the day and peak hours of traffic for a typical weekday. **Table 4** summarizes the total Phase 1 and Phase 2 trip generation for the proposed development during the day and peak hours of traffic for a typical weekday. **Table 5** summarizes the total Phase 1, Phase 2, and Phase 3 trip generation for the proposed development during the day and peak hours of traffic for a typical weekday.

Time Deried	Day			AM Peak Hour			PM Peak Hour		
Time Perioa	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
Parcel A (ITE LUC 770)	3,227	3,227	6,454	617	109	726	170	484	654
Parcel B (ITE LUC 770)	1,218	1,217	2,435	193	34	227	59	169	228
Parcel C (ITE LUC 820)	2,055	2,055	4,110	60	37	97	171	186	357
Parcel D (ITE LUC 770)	2,829	2,828	5,657	541	96	637	150	427	577
Parcel E (ITE LUC 820)	3,175	3,174	6,349	91	55	146	268	291	559
Parcel F (ITE LUC 820)	4,905	4,905	9,810	136	84	220	420	455	875
TOTAL TRIPS	17,409	17,406	34,815	1,638	415	2,053	1,238	2,012	3,250

 Table 3: Trip Generation Summary – Phase 1

Time Period	Day		AM Peak Hour			PM Peak Hour			
	Enter	Exit	Total	Enter	Exit	Total	Enter	Exit	Total
Parcel A (ITE LUC 770)	3,227	3,227	6,454	617	109	726	170	484	654
Parcel B (ITE LUC 770)	1,218	1,217	2,435	193	34	227	59	169	228
Parcel C (ITE LUC 820)	2,055	2,055	4,110	60	37	97	171	186	357
Parcel D (ITE LUC 770)	2,829	2,828	5,657	541	96	637	150	427	577
Parcel E (ITE LUC 820)	3,175	3,174	6,349	91	55	146	268	291	559
Parcel F (ITE LUC 820)	4,905	4,905	9,810	136	84	220	420	455	875
Parcel G (ITE LUC 220)	1,523	1,523	3,046	47	187	234	185	99	284
Parcel H (ITE LUC 230)	277	276	553	8	37	45	35	17	52
Parcel I (ITE LUC 230)	412	412	824	11	54	65	51	25	76
Parcel J (ITE LUC 230)	271	270	541	7	37	44	34	17	51
Parcel K (ITE LUC 230)	245	244	489	7	33	40	31	15	46
Parcel L (ITE LUC 210)	318	318	636	13	37	50	40	24	64
Parcel N (ITE LUC 230)	176	176	352	5	25	30	23	11	34
Parcel O (ITE LUC 230)	299	299	598	8	40	48	38	18	56
Parcel P (ITE LUC 210)	403	403	806	16	46	62	51	30	81
Parcel Q (ITE LUC 230)	325	324	649	9	43	52	41	20	61
Parcel R (ITE LUC 230)	251	250	501	7	34	41	31	16	47
Parcel T (ITE LUC 520)	58	58	116	22	18	41	29	35	65
Parcel U (ITE LUC 230)	316	316	632	9	42	51	40	19	59
TOTAL TRIPS	22,283	22,275	44,558	1,807	1,048	2,856	1,867	2,358	4,226

 Table 4: Trip Generation Summary – Phase 1 & Phase 2

			Dav					DM Dook Hour		
Time Period				Peak H	our	PN	Реак Н	our		
	Enter	Exit	Iotal	Enter	Exit	Iotal	Enter	Exit	Iotal	
Parcel A (ITE LUC 770)	3,227	3,227	6,454	617	109	726	170	484	654	
Parcel B (ITE LUC 770)	1,218	1,217	2,435	193	34	227	59	169	228	
Parcel C (ITE LUC 820)	2,055	2,055	4,110	60	37	97	171	186	357	
Parcel D (ITE LUC 770)	2,829	2,828	5,657	541	96	637	150	427	577	
Parcel E (ITE LUC 820)	3,175	3,174	6,349	91	55	146	268	291	559	
Parcel F (ITE LUC 820)	4,905	4,905	9,810	136	84	220	420	455	875	
Parcel G (ITE LUC 220)	1,523	1,523	3,046	47	187	234	185	99	284	
Parcel H (ITE LUC 230)	277	276	553	8	37	45	35	17	52	
Parcel I (ITE LUC 230)	412	412	824	11	54	65	51	25	76	
Parcel J (ITE LUC 230)	271	270	541	7	37	44	34	17	51	
Parcel K (ITE LUC 230)	245	244	489	7	33	40	31	15	46	
Parcel L (ITE LUC 210)	318	318	636	13	37	50	40	24	64	
Parcel N (ITE LUC 230)	176	176	352	5	25	30	23	11	34	
Parcel O (ITE LUC 230)	299	299	598	8	40	48	38	18	56	
Parcel P (ITE LUC 210)	403	403	806	16	46	62	51	30	81	
Parcel Q (ITE LUC 230)	325	324	649	9	43	52	41	20	61	
Parcel R (ITE LUC 230)	251	250	501	7	34	41	31	16	47	
Parcel S (ITE LUC 210)	683	682	1,365	26	77	103	86	50	136	
Parcel T (ITE LUC 520)	58	58	116	22	18	41	29	35	65	
Parcel U (ITE LUC 230)	316	316	632	9	42	51	40	19	59	
Parcel V (ITE LUC 210)	418	418	836	16	48	64	53	31	84	
Parcel W (ITE LUC 210)	573	573	1,146	22	65	87	72	42	114	
Parcel X (ITE LUC 210)	593	592	1,185	23	67	90	74	44	118	
Parcel Y (ITE LUC 210)	569	568	1,137	22	64	86	72	42	114	
Parcel Z (ITE LUC 210)	583	583	1,166	22	66	88	73	43	116	
Parcel AA (ITE LUC 210)	457	457	914	18	52	70	58	34	92	
Parcel BB (ITE LUC 210)	564	563	1,127	21	64	85	71	42	113	
Parcel CC (ITE LUC 210)	13	13	26	2	3	5	2	1	3	
Parcel DD (ITE LUC 210)	443	442	885	17	51	68	56	33	89	
TOTAL TRIPS	27,179	27,166	54,345	1,996	1,605	3,602	2,484	2,720	5,205	

 Table 5: Trip Generation Summary – Phase 1, Phase 2 & Phase 3

The complete calculation results are contained in Attachment C.

The trip distribution was based on the current and historical distribution observed on Maricopa-Casa Grande Highway, and the available access to the site relative to its anticipated market area. **Figure 8** through **Figure 10** provides the proposed site 2020 (Phase 1), 2025 (Phase 1 & 2), and 2030 (Phase 1, 2 & 3) morning and evening peak hour turning movement volumes, respectively. **Figure 11** through **Figure 13** provides the total future anticipated 2020, 2025, and 2030 with proposed site morning and evening peak hour turning movement volumes, respectively.

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Figure 8: Site (Phase 1) Traffic Volumes – AM & PM Peak Hours

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Figure 9: Site (Phase 1 & 2) Traffic Volumes – AM & PM Peak Hours

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Figure 10: Site (Phase 1, 2 & 3) Traffic Volumes – AM & PM Peak Hours

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Figure 11: 2020 with Site (Phase 1) Traffic Volumes – AM & PM Peak Hours

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Figure 13: 2030 with Site (Phase 1, 2 & 3) Traffic Volumes – AM & PM Peak Hours

TRAFFIC ANALYSIS

The ability of a transportation system to transmit the transportation demand is characterized as its level-of-service (LOS). Level-of-service is a rating system from "A", representing the best operation with the least delay, to "F", representing the worst operation with the greatest delay. Typically, level-of-service "D" or "E" is considered the minimum acceptable operation. The appropriate reference for level-of-service operation is the Highway Capacity Manual, published by the Transportation Research Board – a division within the National Research Council which serves both the National Academy of Engineering.

This manual considers the average delay per vehicle as the measure to determine the level-of-service for both signalized and unsignalized intersections. For signalized intersections and for multi-way stop intersections, the delay and level-of-service are calculated for the intersection, each approach, and each turning movement. For unsignalized intersections the level-of-service is defined for each minor movement for two-way stop controls, and is not defined for the major street approaches or for the entire intersection. **Table 6** lists the level-of-service criteria for both signalized and unsignalized intersections as stated in the Highway Capacity Manual.

	AVERAGE DELAY (seconds-per-vehicle)						
LEVEL-OF-SERVICE	UNSIGNALIZED	SIGNALIZED					
A	<u>≤ 10</u>	≤ 10					
В	> 10 to 15	> 10 to 20					
С	> 15 to 25	> 20 to 35					
D	> 25 to 35	> 35 to 55					
Е	> 35 to 50	> 55 to 80					
F	> 50	> 80					

Table 6: Level-of-Service Criteria for Intersections

Synchro software was utilized to calculate the level-of-service and delay. **Figure 14** through **Figure 16** provides the total future anticipated 2020, 2025, and 2030 with proposed site level-of-service analysis for the morning and evening peak hours, respectively. **Attachment D** contains the level-of-service analysis output sheets.

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Figure 15: 2025 with Site (Phase 1 & 2) Level-of-Service – AM & PM Peak Hours

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Based on the results of the traffic analysis, and as illustrated in **Figure 14** through **Figure 16** above, the following traffic control and auxiliary lane improvements are recommended in order to provide adequate level-of-service:

Year 2020 with Phase 1 Site

Anderson Road / Maricopa-Casa Grande Highway

- Realign Anderson Road north of MCGH to form 4-way intersection
- Provide traffic signal control with split phase timing for the northbound and southbound Anderson Road approaches
- Provide two eastbound through lanes on MCGH
- Provide eastbound channelized "free-flow" right turn lane on MCGH
- Provide westbound left-turn deceleration lane on MCGH
- Provide northbound dual left-turn lanes on Anderson Road

Anderson Road / Access 1

- Provide traffic signal control
- Provide southbound right-turn deceleration lane on Anderson Road
- Provide northbound left-turn deceleration lane on Anderson Road
- Provide eastbound dual left-turn lanes on Access 1 road

Anderson Road / Access 2

- Provide traffic signal control
- Provide southbound right-turn deceleration lane on Anderson Road
- Provide northbound left-turn deceleration lane on Anderson Road
- Provide eastbound dual left-turn lanes on Access 2 road

Year 2025 with Phase 1 & 2 Site

Anderson Road / Maricopa-Casa Grande Highway

- Provide two westbound through lanes on MCGH
- Provide westbound dual left-turn lanes on MCGH
- Provide northbound right-turn deceleration lane on Anderson Road
- Provide southbound left-turn deceleration lane on Anderson Road

Anderson Road / Access 1

- Provide two northbound and southbound through lanes on Anderson Road
- Provide southbound left-turn deceleration lane on Anderson Road
- Provide westbound left and right-turn deceleration lanes on Access 1 road

Anderson Road / Access 3

- Provide stop sign control on the westbound Access 3 road approach
- Provide southbound left-turn deceleration lane on Anderson Road
- Provide westbound left and right-turn deceleration lanes on Access 3 road

Year 2030 with Phase 1, 2 & 3 Site

Anderson Road / Access 1

- Provide three northbound and southbound through lanes on Anderson Road

Anderson Road / Access 3

- Provide two-way left-turn lane median or westbound to southbound acceleration lane on Anderson Road

In addition to the above improvements, a secondary east-west connection to Anderson Road is assumed to provide additional access to the southern parcels. The exact location and alignment of this connection is currently unknown. For the purposes of this analysis, this additional connection is anticipated to be located south of Access 3. It is anticipated that stop sign control on the minor approaches will provide adequate traffic operation.

In order to provide adequate level-of-service and mitigate the LOS "F" movements at the Anderson Road / Maricopa-Casa Grande Highway intersection, it is also necessary to provide a new regional east-west through road south of MCGH to provide alternate routes as identified in the *Area Transportation Plan*.

Figure 17 through **Figure 19** provides the recommended roadway cross sections for future anticipated 2020, 2025, and 2030 with proposed site traffic conditions, respectively.

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Figure 17: 2020 with Site (Phase 1) Recommended Roadway Cross-sections

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Figure 18: 2025 with Site (Phase 1 & 2) Recommended Roadway Cross-sections

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Figure 19: 2030 with Site (Phase 1, 2 & 3) Recommended Roadway Cross-sections

Please contact me at (480)503-2250, extension 125 if you have any questions or would like to discuss this memorandum.

ATTACHMENTS

- A. Previous City of Maricopa Comments
- B. Pertinent Excerpts from Previous Report
- C. Proposed Development Trip Generation
- D. Level-of-Service Analysis Output Sheets



Expires:6/30/2020

ATTACHMENT A PREVIOUS CITY OF MARICOPA COMMENTS





dspermits@maricopa-az.gov

Maricopa, AZ 85138

(520) 568-9098

Deficiency Notice

08/29/2017

34. [Page 20][0034] See comment #21	Anderson Russell PAD - 08/14/2017 Citizen Participation Plan 2017 -06-12.pdf
35. [Page 20][0035] See comment #22	Anderson Russell PAD - 08/14/2017 Citizen Participation Plan 2017 -06-12.pdf
36. [Page 20][0036] change:Ryan Wozniak, Planner, at 520-31	6-6933 Anderson Russell PAD - 08/14/2017 Citizen Participation Plan 2017 -06-12.pdf
37. [Page 21][0037] Apply all comments from Exhibit F.1	Anderson Russell PAD - 08/14/2017 Citizen Participation Plan 2017 -06-12.pdf
38. [Page 20][0038] Case # PAD16-04	Anderson Russell PAD - 08/14/2017 Citizen Participation Plan 2017 -06-12.pdf
39. [Page 15][0039] At minimum, the narrative should include: locationSite acreageExisting use(s)Proposed uses within la planOther exhibit to include:Land Use Plan (Exhibit G)	ite Anderson Russell PAD - 08/14/2017 nd use Citizen Participation Plan 2017 -06-12.pdf
Requirement: TIA	Version 1 Received 06/19/2017 Deficient
70. [Page 1][0070] Please expand to include all things discusse about the revised TIA:Set expectations for the phased development correspondence with the Phased Development exhibitSet ex- for improved intersections per phase (or sub-phase, if applicable)Pavement markings, traffic controls, etc.Provide recommendations for all arterial sections (including the off-s Anderson Rd and Teel Rd to ensure adequate LOS associa development)Mindful of Anderson Rd half-street, and section existing ROW immediately impacted by the development)T questions are best handled by Kerry Osborn, Engineering F Examiner at 520-326-6839	d previously AR - Rev TIA 5.25.17.pdf 08/14/2017 lopment in xpectations ite ted with the ns with echnical lans



MEMO		Planning Division
То:	Jordan Rose, Rose Law Kelly Hall, PMC	
From:	Ryan Wozniak, Planner	
Through:	Martin Scribner, Development Services Director Kazi Haque, Zoning Administrator	
Date:	August 2, 2017 (Submittal #4)	
RE:	PAD16-04 Anderson Russell Planned Area Development - D	eficiencies

ANDERSON RUSSELL PAD REQUEST

Rezone to Planned Area Development (PAD) for 776 acres located generally south of the intersection of Anderson Road and Maricopa-Casa Grande Highway.

REVIEWS TO DATE – UNRESOLVED DEFICIENCIES WITH PROPOSAL

DEPENDENCE ON OUTSIDE AGENCIES

- 1. Will-Serve Letters (per Subdivision Ordinance Article 14-5)
 - a. Provide copies of all. My records only indicate I have one from the CG School District.
 - b. Particularly concerning is Global Water, which by details of the latest narrative (revised 6/12/17): Water and wastewater service will be provided by Global Water Resources. The applicant has been working closely with Global in its process to obtain a CC&N from the Arizona Corporation Commission for service to this area. (p. 3)
- **2.** UPRR Commitment
 - a. At the very least, it is to be stipulated that *explicit consent* shall be provided at the time of making Pre-Plat application to allow for the full arterial widening
- **3.** Floodplain Improvements are required
 - a. As indicated in previous reviews, per letter from Pinal County Department of Public Works dated February 13, 2015: Based on the location of this property, it appears that levees have been constructed to control the flooding impacting this property. As the City of Maricopa's Floodplain Administrator, Pinal County would not permit or approve of the use of levees to contain the floodplains. Development of this property would require that an alternate solution be used to mitigate the flood hazard or that the proposed improvements would be built with the assumption that the levee failed to contain the flows in Santa Cruz



MEMO

Planning Division

Wash. In other words, buildings cannot be constructed in reliance on these uncertified structures. (Comment #1)

- b. At the very least, it is to be stipulated that designed improvements be supplied with the Pre-Plat application
- **4.** An Avigation Easement will be required
 - a. Ak-Chin has indicated officially that the existing Land Plan Concept (C3) has two remaining issues, one of which is an avigation easement and building height limitations that appear appropriate with proximity to an airport.

INFRASTRUCTURE AND SERVICES

- **5.** Improvement plans should have no less than the level of commitment that was outlined as part of the annexation process, as documented in <u>ORD 13-01, Exhibit G</u>
- **6.** The numbers of residential units have increased since the Fiscal Impact Analysis prepared for ANX13-01, further cause for considering public liabilities of the development proposal compounded by the remoteness of the property based on existing conditions.
- **7.** Internal circulation proposed routes a significant amount of estimated traffic volume adjacent to the Community Park and school site unnecessarily.
- **8.** The Transportation Impact Analysis describes phasing by a percentage of development while the Phasing Plan Exhibit shows phasing by geography. These commitments lack coordination. Therefore, the Phasing Plan is technically incomplete in staff's judgment. Furthermore, the TIA speaks to only the signalization of Anderson Road at Maricopa-Casa Grande Highway. Anderson Road half-street condition is not defined.
 - a. In order for staff to make the Required Findings (as discussed previously), per Zoning Code Sec. 510.04.C, the TIA should establish the basic framework that can be refined at the time of Pre-Plat Application, per Subdivision Ordinance Sec. 14-7-3-D (i.e. future on-site related traffic, on-site and off-site LOS to be evaluated at each proposed phase, all traffic signalization, etc.)
 - b. Consider the dry crossings/bridges required at all points where roads intersect with washes.
 - c. Consider improvements across the MSIDD eastward, not making it a barrier to the multi-modal circulation for the community.
- **9.** Clarify the Emergency Access Plan to the satisfaction of MPD, MFD, and the City Engineer.

LAND USES AND DEVELOPMENT CHARACTER

10. Mixed Use designations are relatively undefined, missing commitments to building form, required conditions for the mixing of uses, and undefined public realm through street character and street network characteristics (consider on-street parking). Provide requirements that avoid zoning for high density residential mischaracterized as mixed use. Consider a transition of intensity based on the surrounding characteristics, such as proximity to the approach of the Ak-Chin Airport and surrounding land uses (as proposed). – Accurately describing this will improved future TIA refinements (upon Pre-Plat), as well.


MEMO

Planning Division

- **11.** Useable Open Space has been proposed to be lowered adjacent to Condominium/Apartment Developments (@40%) and adjacent to the Specialty Product Development (@ 0%) since adjacent to the Community Park/School.
 - a. 40% O/S adjustment This was a concern for the PRL Committee. The Committee felt as though a reduction was the opposite direction for an adjustment. Staff holds to at least the minimum per code of 60% Useable Open Space, per Subdivision Ordinance Sec. 14-6-4-B.
 - b. 0% O/S adjustment This is concerning due to the accessibility of the O/S is in question. Refinement options: Refine with a commitment to a short walk to useable O/S. Refine with a commitment that the adjacent parcels will not be developed ahead of the park being improved and accepted by the City. Refine by not dividing the land use away from the park with a street with significant volume of vehicular traffic.
- **12.** Nature Preserve OS: The intentions of the Nature Preserve OS lack the commitments necessary to ensure connectivity across the washes when flowing. Additionally, commitments are lacking to know whether these spaces qualify as "useable open space" as required in the Subdivision Ordinance (Sec. 14-6-4-B, which requires minimum of 60% of all O/S be "useable"). The current proposal for allocation of open space relies on a significant amount of Nature Preserve OS (this may make for future development challenges). Refine with a detail as to whether the O/S will include bridges and amenity commitments at flood elevations per the Subdivision Ordinance.

SUBMITTAL

- **13.** Update:
 - a. Exhibits to reflect all changes in land use plan
 - b. Refined street sections (including all street conditions in all phases, consider mixed use character)
 - c. Provide addendum to TIA providing details for the missing improvements, intersections, and associate traffic improvements with development phasing plan (geographically) this is serving as the required "Development Schedule" per the Zoning Code
 - d. Narrative images to reflect changes to the exhibits, to codify design considerations of the mixed use designations (see comment #10)
 - e. Will-Serve Letters (see comment #1)

Respectfully,

Ryan Wozniak, AICP

Planner Planning and Development Services City of Maricopa <u>ryan.wozniak@maricopa-az.gov</u> <u>www.maricopa-az.gov</u>





Anderson Russell Property PAD14-01 | 2nd Review

Reviewed August 11, 2015 by Richard Wallace | Jacobs Engineering | 480-440-8251

Submittal Table	
Item	Engineering Review Status
Rezoning Request Narrative (July 8, 2015)	Comments as noted.
Preliminary Drainage Report (June 16, 2015)	Comments as noted.
Phase 1 Environmental	Not resubmitted
Traffic Impact Analysis	Not resubmitted
Water & Wastewater Reports	Not resubmitted
ALTA Land Survey	Not resubmitted

PAD Rezoning Request Narrative

- 1. General Comment:
 - a The City's Area Transportation Study should be concluded within the next 12 months. Recommendations from this study should be incorporated into the site development and reflected in the associated traffic analysis required of the applicant for this site.
 - b Site reserved for public safety facilities should not be located within floodplain or floodplain issues appropriately mitigated. Site should be available and provided when public safety needs warrant, which may be prior to Phase 2 where it is shown.
- 2. Section 3j. Anderson Road Improvements:
 - a Anderson Road is included with the City's Area Transportation Study currently underway.
 - b Applicant should provide ultimate half-street improvements on all existing and proposed roadways adjacent to the site. This includes Anderson Road, Russell Road and Teel Road.
 - c Proposed phasing of Anderson Road improvements should be done from outside-in to minimize reconstruction of curbs and modifying drainage and parkway landscaping.
- 3. Exhibit J Master Street Plan:

- a Show where cross section "F" applies on the map.
- b Roadway A is referred to as an arterial, but this roadway appears to function as a major collector (4 lanes).
- c Roadway E is proposed as one-way. This does not provide for efficient circulation.
- d Indicate whether the future access to Santa Cruz Ranch will be all-weather.
- e Anderson Road and Val Vista Boulevard are anticipated to be Regionally Significant Roads (RSR) and, therefore, an ultimate 6-lane facility within 200' of right-of-way. Add typical section to reflect this.

Preliminary Drainage Report

- 1. Pinal County has indicated the levee along the east boundary is not FEMA certified and this property should be designed assuming the levee is not in place. Address this in report.
- 2. Add Q₁₀₀ flow rate to Drainage Map (Figure 5) for offsite flow crossing Val Vista Blvd (Teel Road) at southeast corner of site and at Anderson Road south of UPRR tracks. Perimeter streets should be designed as all-weather roads.
- 3. A Floodplain Use Permit will be required for any work within the floodplain.



Anderson Russell Property – PAD14-01| 1st Review Reviewed February 12, 2015 by Laura Wallis | 520-316-6839

General Comments

- **1.** Supporting documentation such as the Drainage Report, Phase 1 Environmental Assessment, Traffic Impact Analysis, Water Report, and Wastewater Report will not be approved with the PAD. Comments given below are intended to help in future development of the area. No rights shall be entitled with these documents, and no content shall be accepted and/or approved as part of this PAD approval.
- 2. The Developer will need to coordinate with the utility companies to verify space for required facilities, such as substations, have been allocated.

PAD Rezoning Request Narrative:

- 1. Police, Fire, and school facilities are identified as part of the land use, but are not identified on the land use exhibits. Developer will need to coordinate with the City and school district to identify land to be dedicated for these purposes.
- 2. The development agreement between Santa Cruz Ranch and the County does not alter the requirements identified by the City of Maricopa regarding the construction of half street and other improvements which will be the responsibility of the Anderson Russell Property.
- 3. The City's Area Transportation Study will be concluded within the next 18 months. Assumptions within the narrative as to the future findings may be premature. Please remove all references to future reports and studies.
- 4. Further coordination with the Ak-Chin airport appears to be required. Preliminary feedback from airport staff indicates that the exhibits overlaying airport facilities, protection zones, and approach surfaces may not be accurate. Please provide communication from the airport indicating that the information provided within this report matches the existing airport documentation, including current airport development information.
- 5. The Existing Ownership Map exhibit includes Killian C Max TR and Dobson Rev Trus as property owners; however they are not mentioned anywhere else. Please explain the relationship and their participation in this project.
- 6. Round Abouts are identified in Exhibit O, but are not mentioned in the Traffic Study. This level of detail may be too specific for the PAD Narrative, and should be removed.

Drainage Report:

7. Verify that the report references and studies the correct location. Page 1 appears to place the development in too many sections. Verify that the analysis was done for the correct area.

8. Make sure that the land use/layout shown within this report matches the PAD narrative and exhibits.

Phase 1 Environmental:

9. Verify that all wells on the site are identified and listed as either capped or uncapped. The City will not approve any documents beyond the PAD unless all wells are in compliance with the Arizona State Statute (Article 8, R12-15-822) which requires them to either be capped or abandoned per ADWR guidelines.

Traffic Impact Analysis:

- 10. Improvement of the intersection at Anderson Road and Maricopa Casa Grande Highway (MCGH) has many limitations. Before submittal of improvement plans, there will need to be communication between the developer, the rail road, the irrigation district, the flood control district, potentially the Ak-Chin airport, and any other concerned entities to discuss design constraints.
- 11. The roadway connection identified between the western and eastern portions of the property (over the north east corner of the State land) will conflict with the existing irrigation canal. Before submitting improvement plans, the developer will need to coordinate with the irrigation district to verify feasibility of that alignment. The roadway may not be able to be located in that area, in which case the developer may need to discuss easements across state land.
- 12. The City does not agree with the peak hour traffic assumptions. There are likely to be many households with two cars per dwelling unit traveling during peak hours. Using a Trip Rate of less than one, and then further reducing it for people who work within the community is an unrealistic condition.
- 13. When modeling the intersection at Anderson Rd and MCGH, please be aware that the current alignment of the intersection has the north and south bound roadways offset. This offset requires all traffic on Anderson to turn onto MCGH before turning again onto the continuation of Anderson. That turning movement is not currently reflected in the data presented in the report.
- 14. The City of Maricopa is working on an updated Area Transportation Study, which will be concluded in approximately 18 months. Please include the findings of that report within future traffic studies for this site.
- 15. When calculating background traffic, please include surrounding future communities which are likely to develop within the same time frame.
- 16. The City does not accept the Traffic Impact Analysis as part of the PAD, however, the information and assumptions provided are deficient for any subsequent submittals and approvals. Please revise before resubmitting.



Wastewater Report and Water:

- 17. Before improvement plans are submitted, the developer will need to coordinate with the rail road and the irrigation district to verify the ability to cross their facilities as noted on Anderson Road.
- 18. Verify that all data referenced in these reports matches the land use identified in the approved PAD.
- 19. The water system must be looped. It appears the proposed layout is not always looped (P-8 as an example). Please verify that all future submittals include completely looped systems.

Citizen Participation Plan:

20.Numerous exhibits are referenced within the report, but are not included or clearly labeled within the report. Please clarify and update.

ALTA/ASCM Land Title Survey:

- 21. The ALTA has expired. Please submit an updated ALTA with updated line work per comment 25.
- 22. Correct the Vicinity Map to reflect the correct spelling of Peter and Nall Road and update the rail road to Union Pacific.
- 23. Identify the specific basis of bearing on the cover sheet.
- 24. The zoning information will come from the City of Maricopa, not the County. Please revise statement on cover and list current zoning.
- 25. Typical on sheets 2-4 The line work is too dark to clearly read. Use differing line weights and styles to clearly identify the features and dimensions represented on this document.
- 26. Why is overhead electric listed as OHW instead of OHE? Does this have a specific meaning?
- 27. Irrigation canal is called out with 18" concrete headwall. What does the dimension 18" represent? The thickness of the concrete? The headwall is much larger than 18" across.



ATTACHMENT B PERTINENT EXCERPTS FROM PREVIOUS REPORT



ANDERSON RUSSELL MARICOPA CASA GRANDE HIGHWAY / ANDERSON ROAD MARICOPA, ARIZONA

TRAFFIC IMPACT ANALYSIS

August 2014

► Prepared for:



Coe & Van Loo Consultants, Inc. 4550 North 12th Street Phoenix, Arizona 85014 Phone: (602) 264-6831 Fax: (602) 285-4731

Prepared by:



TRACE Consulting, LLC. 1201 E Jefferson Street, Suite 3 Phoenix, Arizona 85034 Phone: (602) 680-8264



EXECUTIVE SUMMARY

The proposed Anderson Russell development is located south of the Casa Grande Highway along Anderson Road in Maricopa, AZ. The proposed site is a multi-use development including retail, office and single family housing. It is planned to include 2,080 single family homes, 291,000 square feet (sf) of retail and 872,000 sf of office space at build-out. The development is planned to be constructed over the next 15 years. 25% of the development is projected to be completed by 2015, 50% of the development is planned to be completed by 2020, 75% of the development is projected to be completed by 2025 and 100% of the development is planned to be completed by 2030. While the study assumes 5-year intervals beginning in 2015, the analysis is based on development levels. The recommendations are based on the level of development analyzed in 25% intervals and therefore the recommended improvements are connected to the amount of development completed and not to specific year of improvement.

The site is projected to generate 3,098 AM and 4,311 PM peak hour trips at full build-out. Currently, the majority of the traffic will travel to Casa Grande Highway to the north and State Route 84 to the south as there are few other routes available to service the land. In the future, Val Vista Parkway as well as Hassayampa Freeway will provide other east-west opportunities for mobility. These are both shown on the 2009 Maricopa Association of Governments (MAG) and Central Arizona Association of Governments (CAAG) Regional Council's "Transportation Framework Recommendations". Within that plan, Anderson Road is planned as a 4-lane roadway in the short-term, with it becoming a 6-lane facility in the full build-out condition. The evaluation also included a review of the 2008 City of Maricopa Regional Transportation Plan (RTP) and the 2007 City of Casa Grande Small Area Transportation Plan. It should be noted that the RTP will be updated over the next year and this information can thus be incorporated into the updated plan.

There are three accesses from Anderson Road planned to serve the site. The study area includes the following intersections and accesses:

- Maricopa Casa Grande Highway/ Anderson Road
- Anderson Road/ SR 84
- Anderson Road/ Access 1
- Anderson Road/ Access 2

• Anderson Road/ Access 3

This primary purpose of this report is to identify infrastructure improvements that are directly related to the development in order to support traffic operations. As identified in the future volume projections for Anderson Road and Maricopa Casa Grande Highway, the area surrounding Maricopa and Casa Grande are projected to experience high growth between now and full build-out of this area. To provide a reasonable analysis for this site and to identify specific improvements required for the site, future annual growth is based on the historic growth rate of 3.71%. When combined with the site generated traffic, this approach yields 2030 AADT volumes of 32,000 vehicles per day (vpd) along Anderson Road, 37,000 vpd on the Maricopa Casa Grande Highway west of Anderson Road and 17,000 vpd on Maricopa Casa Grande Highway east of Anderson Road.

These volumes are lower than those projected by the additional studies and master plans evaluated for this study. The purpose in utilizing the historic growth is to demonstrate the specific improvements necessary to support the site and not the improvements that are planned to support the entire region developing. The recommended improvements are summarized in the Table ES-1 and Exhibit 13. As the development ensues, updates should monitor that the traffic is being accommodated by these recommended improvements. The logical time would be in between phases.

Table ES-1: Recommended Improvements:

	2015 (Site Opening)	2025 (75%)	2030 (100%)
Maricopa Casa Grande Highway/ Anderson Road	 Signalize Intersection Build signal with separate left turn lanes for each approach and separate EB and NB right turn lanes 	 Improve signal to include dual westbound and northbound left turn lanes Improve Anderson Road and Maricopa Casa Grande Highway to include 2 acceptance lanes for dual lefts Change phasing for eastbound left to protected plus overlap The northbound right turn lane becomes a drop lane 	 Improve signal to include two eastbound and westbound through lanes Improve signal to include a eastbound channelized free right turn
Anderson Road/ Access 1	 Build access with separate left turn lanes for eastbound and westbound approaches Improve Anderson Road to include northbound and southbound left turn deceleration lanes 	 Improve signal to include dual eastbound left turn lanes Improve Anderson Road to include 2 northbound and southbound through lanes Improve signal to include westbound and southbound right turn lanes 	• The third southbound lane along Anderson Road becomes a drop lane at this access
Anderson Road/ Access 2	 Build access with separate eastbound left and right turn lanes Improve Anderson Road to include northbound left turn deceleration lane 	Build additional north and southbound through lane	• No additional improvements
Anderson Road/ Access 3	 Build access with separate westbound left and right turn lanes Improve Anderson Road to include southbound left turn deceleration lane 	 Signalize intersection Build signal with separate left turn lanes for each applicable approach 	• No additional improvements
Maricopa Casa Grande Highway	 No additional improvements excepting those related to the newly signalized intersection 	• Widen to include two westbound acceptance lanes for the dual northbound left turns	• Widen to a 4-lane facility
Anderson Road	• Improve to a 2-lane roadway with 1 lane in each direction and a two way left turn lane from Maricopa Casa Grande Highway to Access 3	• Improve to a 4-lane roadway with 2 lanes in each direction and a two way left turn lane from Maricopa Casa Grande Highway to Access 3	• Add a third southbound lane from Maricopa Casa Grande Highway to Access 1 for a free right turn movement at the signal, this lane is a drop lane at Access 1





3.0 PROJECT RELATED TRIP GENERATION AND DISTRIBUTION

3.1 Trip Generation

The Institute of Transportation Engineers (ITE) *Trip Generation* (9th Edition) handbook was used to estimate trips for the proposed site. Table 2 shows the AM and PM peak hour trips generated. The site is planned to include 2,080 Single Family Homes, 291,000 sf of retail space and 872,000 sf of office space. A 10% internal trip reduction was included for the site. This represents trips that stay within the development due to school, resident to resident and local retail or other business trips. The site is projected to generate 3,098 AM and 4,311 PM peak hour trips at full build-out with 2,788 AM and 3,880 PM trips after a 10% internal trip reduction was applied.

	Size	Land Use	Trip Rate	Trips	% IN	% OUT	IN	OUT
AM								
Retail	291,000	820	1	291	61%	39%	160	102
Office	872,000	770	1.43	1247	84%	16%	942	180
Single Family	2080	210	0.75	1560	25%	75%	351	1053
Total			0	3098			1453	1335
			PM					
Retail	291,000	820	3.73	1085	49%	51%	479	498
Office	872,000	770	1.29	1125	23%	77%	233	779
Single Family	2080	210	1.01	2101	63%	37%	1191	700
Total				4311			1903	1977

Table 2: Site Trip Generation

3.2 Trip Distribution Assignment

Project site traffic was applied to the origin-destination (O-D) for the site. Based on current distribution of traffic at the counted intersections the following O-D was determined.

- 5% to/from the south on Anderson Rd
- 15% to/from the east on Maricopa Casa Grande Highway
- 80% to/from the west on Maricopa Casa Grande Highway.

For the traffic study, modifications to the existing O-D are necessary because of the residential and commuter nature of the traffic flow generated by the site. Maricopa Casa Grande Highway is

a commuter route between Casa Grande and Maricopa with commuter traffic traveling to the southeast in the AM and northwest in the PM peak period. Taking this commuter traffic into account the O-D that is applied in the study is:

- 5% to/from south on Anderson Road
- 30% to/from the east on Maricopa Casa Grande Highway
- 65% to/from the west on Maricopa Casa Grande Highway.

Once Val Vista Parkway is completed, it is projected that the traffic utilizing the southern access from Anderson Road will access this roadway via Val Vista Parkway.

The O-D utilized for the study is shown in Exhibit 5. Trip assignment for 2015, 2020 and 2025 are shown in Exhibit 6. By 2030 with the full build-out of the site, additional capacity is required for the site to function at acceptable LOS, therefore Val Vista Parkway is recommended to be constructed from the south of the site to its eastern connection to Maricopa Casa Grande Highway. This connection allows for site trips headed to the east to utilize this additional access and Exhibit 7 shows the trip assignment for 2030.







ATTACHMENT C PROPOSED DEVELOPMENT TRIP GENERATION



	·			
	ł	ANDERSON	RUSSELL	
	ł			
ITE LAND USE CATEGORY AND CODE	l	BUSINESS	<u>PARK - 770</u>	
	l	1,000 SQU/	ARE FEET	/
SIZE	 	518.	800	/
	1 '		TRIPS	
	<u>ا</u>	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	<u>ا</u> ــــــــــــــــــــــــــــــــــــ	50%	50%	
NUMBER OF STUDIES	16	1		
AVERAGE SIZE	393			
MINIMUM RATE	5.56	1,443	1,442	2,885
AVERAGE RATE	12.44	3,227	3,227	6,454
MAXIMUM RATE	27.96	7,253	7,253	14,506
STANDARD DEVIATION	5.61			
EQUATION <u>: T = 10.62 * (X) + 715.61</u>	$R^2 = 0.89$	3,113	3,112	6,225
LARGEST OF AVERAGE OR EQUATION	[]	3,227	3,227	6,454
AM PEAK HOUR ADJACENT STREET	'	85%	15%	
NUMBER OF STUDIES	20			
AVERAGE SIZE	384	1		
MINIMUM RATE	0.65	286	51	337
AVFRAGE RATE	1.40	617	109	726
MAXIMUM RATE	2.90	1.279	226	1.505
STANDARD DEVIATION	1.32	l		.,
$FOI ATION \cdot I N (T) = 0.97 * LN(X) + 0.49$	$R^2 = 0.86$	597	105	702
LARGEST OF AVERAGE OR EQUATION	Π = 0.00	617	109	726
	<u>_</u>	NA	NA	
NUMBER OF STUDIES	NA			
AV/ERAGE SIZE	NA	1 1		
		ΝΔ	ΝΔ	ΝΔ
		ΝΔ	ΝΔ	ΝΔ
				<u></u> ΝΔ
		ΝΑ		ΝΛ
	·	11A 260/	740/	
MUMPED OF STUDIES		20%	/4%	
		4		
	390	74	044	00E
	0.55	14	211	200
	1.20	1/0	484	654
	2.97	401	1,140	1,541
STANDARD DEVIATION	1.26			
EQUATION: LN (T) = $0.90 * LN(X) + 0.85$	$R^2 = 0.82$	169	481	650
LARGEST OF AVERAGE OR EQUATION	<u> </u>	170	484	654
PM PEAK HOUR GENERATOR	I'	NA	NA	
NUMBER OF STUDIES	NA	1		
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION	[]	NA	NA	NA
				FPS
				GROUP

PROJECT		ANDERSON	RUSSELL	
PARCEL		PARC	ELA	
ITE LAND USE CATEGORY AND CODE		BUSINESS	PARK - 770	
INDEPENDENT VARIABLE		1,000 SQU	ARE FEET	
SIZE		518.	800	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	11			
AVERAGE SIZE	485			
MINIMUM RATE	1.10	286	285	571
AVERAGE RATE	2.56	664	664	1,328
MAXIMUM RATE	5.30	1,375	1,375	2,750
STANDARD DEVIATION	1.96			
EQUATION: LN (T) = 0.83 * LN(X) + 1.94	$R^2 = 0.81$	624	623	1,247
LARGEST OF AVERAGE OR EQUATION		664	664	1,328
PEAK HOUR GENERATOR		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	11			
AVERAGE SIZE	485			
MINIMUM RATE	0.74	192	192	384
AVERAGE RATE	1.29	335	334	669
MAXIMUM RATE	2.28	592	591	1,183
STANDARD DEVIATION	1.27			
EQUATION: LN (T) = 0.99 * LN(X) + 0.27	$R^2 = 0.86$	319	319	638
LARGEST OF AVERAGE OR EQUATION		335	334	669
PEAK HOUR GENERATOR		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
				EPS BROUP

PROJECI	ł		RUSSELL	
	ł		ELB	
ITE LAND USE CATEGORY AND CODE	l	BUSINESS	PARK - 770	
	 	1,000 SQU/	ARE FEET	
SIZE	 		934	/
l IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	1		TRIPS	
L	L'	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	' '	50%	50%	
NUMBER OF STUDIES	16	1		1 1
AVERAGE SIZE	393			L
MINIMUM RATE	5.56	450	450	900
AVERAGE RATE	12.44	1,007	1,007	2,014
	27.96	2,264	2,264	4,528
STANDARD DEVIATION	5.61			
EQUATION: T = 10. <u>62 * (X) + 715.</u> 61	$R^2 = 0.89$	1,218	1,217	2,435
LARGEST OF AVERAGE OR EQUATION	[]	1,218	1,217	2,435
AM PEAK HOUR ADJACENT STREET	<u> </u>	85%	15%	
NUMBER OF STUDIES	20			
AVERAGE SIZE	384	1		1 I
MINIMUM RATE	0.65	89	16	105
AV/FRAGE RATE	1.40	193	34	227
MAXIMIM RATE	2.90	400	70	470
	1.32	l	t	
EOUNTION(1 N (T) = 0.97 * 1 N(X) + 0.49	$P^2 = 0.86$	103	34	227
ADGEST OF AVERAGE OR EQUATION	$\mathbf{K} = 0.00$	193	34	227
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	ΝΔ			·
		1 1		1 I
		ΝΔ	ΝΔ	ΝΔ
			ΝΙΛ	ΝΙΛ
	<u> </u> '		NA	
PM PEAK HOUK ADJAGENI SIKEEI		20%	/4%	ı — – – – – – – – – – – – – – – – – – –
		4		
	396			
	0.55	23	66	89
AVERAGE RATE	1.26	53	151	204
MAXIMUM RATE	2.97	125	356	481
STANDARD DEVIATION	1.26	↓		
EQUATION: LN (T) = $0.90 * LN(X) + 0.85$	$R^2 = 0.82$	59	169	228
LARGEST OF AVERAGE OR EQUATION	<u>'</u>	59	169	228
PM PEAK HOUR GENERATOR	L'	NA	NA	
NUMBER OF STUDIES	NA	1		1
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION	[]	NA	NA	NA
	· · · · · · · · · · · · · · · · · · ·			FPS
				GROUP

PROJECT		ANDERSON	RUSSELL		
PARCEL		PARCEL B			
ITE LAND USE CATEGORY AND CODE	BUSINESS PARK - 770				
INDEPENDENT VARIABLE		1,000 SQU	ARE FEET		
SIZE		161.	934		
			TRIPS		
	RATE	ENTERING	EXITING	SUM	
SATURDAY DAILY		50%	50%		
NUMBER OF STUDIES	11				
AVERAGE SIZE	485				
MINIMUM RATE	1.10	89	89	178	
AVERAGE RATE	2.56	208	207	415	
MAXIMUM RATE	5.30	429	429	858	
STANDARD DEVIATION	1.96				
EQUATION: LN (T) = 0.83 * LN(X) + 1.94	$R^2 = 0.81$	238	237	475	
LARGEST OF AVERAGE OR EQUATION		238	237	475	
PEAK HOUR GENERATOR		NA	NA		
NUMBER OF STUDIES	NA				
AVERAGE SIZE	NA				
MINIMUM RATE	NA	NA	NA	NA	
AVERAGE RATE	NA	NA	NA	NA	
MAXIMUM RATE	NA	NA	NA	NA	
STANDARD DEVIATION	NA				
EQUATION: NOT PROVIDED	NA	NA	NA	NA	
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA	
SUNDAY DAILY		50%	50%		
NUMBER OF STUDIES	11				
AVERAGE SIZE	485				
MINIMUM RATE	0.74	60	60	120	
AVERAGE RATE	1.29	105	104	209	
MAXIMUM RATE	2.28	185	184	369	
STANDARD DEVIATION	1.27				
EQUATION: LN (T) = 0.99 * LN(X) + 0.27	$R^2 = 0.86$	101	101	202	
LARGEST OF AVERAGE OR EQUATION		105	104	209	
PEAK HOUR GENERATOR		NA	NA		
NUMBER OF STUDIES	NA				
AVERAGE SIZE	NA				
MINIMUM RATE	NA	NA	NA	NA	
AVERAGE RATE	NA	NA	NA	NA	
MAXIMUM RATE	NA	NA	NA	NA	
STANDARD DEVIATION	NA				
EQUATION: NOT PROVIDED	NA	NA	NA	NA	
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA	
				EPS GROUP	

		ANDERSON			
THE LAND USE CATEGORY AND CODE					
		1,000 SQU			
SIZE		46.1	74		
			TRIPS		
		ENTERING	EXITING	TOTAL	
WEEKDAY DAILY		50%	50%		
NUMBER OF STUDIES	302				
AVERAGE SIZE	331				
MINIMUM RATE	12.50	289	288	577	
AVERAGE RATE	42.70	986	986	1,972	
MAXIMUM RATE	270.89	6,254	6,254	12,508	
STANDARD DEVIATION	21.25				
EQUATION: LN (T) = 0.65 * LN(X) + 5.83	$R^2 = 0.79$	2,055	2,055	4,110	
LARGEST OF AVERAGE OR EQUATION		2,055	2,055	4,110	
AM PEAK HOUR ADJACENT STREET		62%	38%		
NUMBER OF STUDIES	104				
AVERAGE SIZE	310				
MINIMUM RATE	0.10	3	2	5	
AVERAGE RATE	0.96	27	17	44	
MAXIMUM RATE	9.05	259	159	418	
STANDARD DEVIATION	1.31				
EQUATION: LN (T) = 0.61 * LN(X) + 2.24	$R^2 = 0.56$	60	37	97	
LARGEST OF AVERAGE OR EQUATION		60	37	97	
AM PEAK HOUR GENERATOR		NA	NA		
NUMBER OF STUDIES	NA				
AVERAGE SIZE	NA				
MINIMUM RATE	NA	NA	NA	NA	
AVERAGE RATE	NA	NA	NA	NA	
MAXIMUM RATE	NA	NA	NA	NA	
STANDARD DEVIATION	NA				
EQUATION: NOT PROVIDED	NA	NA	NA	NA	
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA	
PM PEAK HOUR ADJACENT STREET		48%	52%		
NUMBER OF STUDIES	426				
AVERAGE SIZE	376				
	0.68	15	16	31	
AVERAGE RATE	371	82	89	171	
MAXIMUM RATE	29.27	649	703	1.352	
STANDARD DEVIATION	2 74	0.0		.,	
EQUATION: $I N (T) = 0.67 * I N(X) + 3.31$	$R^2 = 0.81$	171	186	357	
LARGEST OF AVERAGE OR EQUATION	<u> </u>	171	186	357	
PM PFAK HOUR GENERATOR		NA	NA		
	NA				
AVERAGE SIZE	NA				
	NA	NA	NA	NA	
AVERAGE RATE	ΝΔ	ΝΔ	ΝΔ	ΝΔ	
	ΝΔ	ΝΔ	ΝΔ	NΔ	
	ΝΔ		INЛ		
		ΝΔ	ΝΔ	ΝΔ	
		NA NA	NA NA	NA NA	
LANGEDI DI AVERAGE UR EQUATION		MIN	INA		
				GEPS	

			ADE EEET		
		1,000 300			
SIZL		40.			
	RATE	ENTERING	EXITING	SLIM	
SATURDAY DAILY		50%	50%	00111	
NUMBER OF STUDIES	123	0070	0070		
AVERAGE SIZE	450				
	16.70	386	385	771	
AVERAGE RATE	49.97	1,154	1,153	2.307	
MAXIMUM RATE	227.50	5.252	5.252	10.504	
STANDARD DEVIATION	22.62	-,	-,		
EQUATION: LN (T) = 0.63 * LN(X) + 6.23	$R^2 = 0.82$	2.839	2.839	5.678	
LARGEST OF AVERAGE OR EQUATION		2,839	2,839	5,678	
PEAK HOUR GENERATOR		52%	48%	ł	
NUMBER OF STUDIES	128				
AVERAGE SIZE	458				
MINIMUM RATE	1.46	35	32	67	
AVERAGE RATE	4.82	116	107	223	
MAXIMUM RATE	18.32	440	406	846	
STANDARD DEVIATION	3.10				
EQUATION: LN (T) = 0.65 * LN(X) + 3.78	$R^2 = 0.83$	275	254	529	
LARGEST OF AVERAGE OR EQUATION		275	254	529	
SUNDAY DAILY		50%	50%		
NUMBER OF STUDIES	77				
AVERAGE SIZE	439				
MINIMUM RATE	4.15	96	96	192	
AVERAGE RATE	25.24	583	582	1,165	
MAXIMUM RATE	148.15	3,421	3,420	6,841	
STANDARD DEVIATION	17.23				
EQUATION: T = 15.63 * (X) + 4214.46	$R^2 = 0.52$	2,468	2,468	4,936	
LARGEST OF AVERAGE OR EQUATION		2,468	2,468	4,936	
PEAK HOUR GENERATOR		49%	51%		
NUMBER OF STUDIES	39				
AVERAGE SIZE	369				
	0.39	9	9	18	
AVERAGE RATE	3.12	71	73	144	
MAXIMUM RATE	12.40	281	292	573	
STANDARD DEVIATION	2.78				
EQUATION: NOT PROVIDED	NA	NA	NA	NA	
LARGEST OF AVERAGE OR EQUATION		71	73	144	
				C EPS	
				· · · · · · · · · · · · · · · · · · ·	

			DUCCELL		
		ANDERSON			
		1,000 SQU/			
SIZE		454.	766		
			TRIPS		
		ENTERING	EXITING	TOTAL	
	10	50%	50%		
NUMBER OF STUDIES	16				
AVERAGE SIZE	393				
	5.56	1,265	1,264	2,529	
AVERAGE RATE	12.44	2,829	2,828	5,657	
MAXIMUM RATE	27.96	6,358	6,357	12,715	
STANDARD DEVIATION	5.61				
EQUATION: T = 10.62 * (X) + 715.61	$R^2 = 0.89$	2,773	2,772	5,545	
LARGEST OF AVERAGE OR EQUATION		2,829	2,828	5,657	
AM PEAK HOUR ADJACENT STREET		85%	15%		
NUMBER OF STUDIES	20				
AVERAGE SIZE	384				
MINIMUM RATE	0.65	252	44	296	
AVERAGE RATE	1.40	541	96	637	
MAXIMUM RATE	2.90	1,121	198	1,319	
STANDARD DEVIATION	1.32				
EQUATION: LN (T) = 0.97 * LN(X) + 0.49	$R^2 = 0.86$	525	93	618	
LARGEST OF AVERAGE OR EQUATION		541	96	637	
AM PEAK HOUR GENERATOR		NA	NA		
NUMBER OF STUDIES	NA				
AVERAGE SIZE	NA				
MINIMUM RATE	NA	NA	NA	NA	
AVERAGE RATE	NA	NA	NA	NA	
MAXIMUM RATE	NA	NA	NA	NA	
STANDARD DEVIATION	NA				
EQUATION: NOT PROVIDED	NA	NA	NA	NA	
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA	
PM PEAK HOUR ADJACENT STREET		26%	74%		
NUMBER OF STUDIES	21				
AVERAGE SIZE	396				
MINIMUM RATE	0.55	65	185	250	
AVERAGE RATE	1.26	149	424	573	
MAXIMUM RATE	2.97	351	1.000	1.351	
STANDARD DEVIATION	1.26		,) ·	
EQUATION: $LN(T) = 0.90 * LN(X) + 0.85$	$R^2 = 0.82$	150	427	577	
LARGEST OF AVERAGE OR EQUATION	10 - 0.02	150	427	577	
PM PEAK HOUR GENERATOR		NA	NA	011	
NUMBER OF STUDIES	NA	107			
AVERAGE SIZE	NA				
	NA	NA	NA	NA	
AVERAGE RATE	NA	NA	NA	NA	
	ΝA	NA	NΔ	NA	
	ΝA	11/3	11/3	11/3	
	ΝΔ	NΔ	ΝΔ	ΝΔ	
		NA NA	NA NA	NA NA	
LANGEOT OF AVENAGE ON EQUATION		11/4	11/4		
				C-EPS GROUP	

PROJECT		ANDERSON	I RUSSELL			
PARCEL		PARCEL D				
ITE LAND USE CATEGORY AND CODE		BUSINESS	PARK - 770			
INDEPENDENT VARIABLE		1,000 SQU	ARE FEET			
SIZE		454.	766			
			TRIPS			
	RATE	ENTERING	EXITING	SUM		
SATURDAY DAILY		50%	50%			
NUMBER OF STUDIES	11					
AVERAGE SIZE	485					
MINIMUM RATE	1.10	250	250	500		
AVERAGE RATE	2.56	582	582	1,164		
MAXIMUM RATE	5.30	1,205	1,205	2,410		
STANDARD DEVIATION	1.96					
EQUATION: LN (T) = 0.83 * LN(X) + 1.94	$R^2 = 0.81$	559	559	1,118		
LARGEST OF AVERAGE OR EQUATION		582	582	1,164		
PEAK HOUR GENERATOR		NA	NA			
NUMBER OF STUDIES	NA					
AVERAGE SIZE	NA					
MINIMUM RATE	NA	NA	NA	NA		
AVERAGE RATE	NA	NA	NA	NA		
MAXIMUM RATE	NA	NA	NA	NA		
STANDARD DEVIATION	NA					
EQUATION: NOT PROVIDED	NA	NA	NA	NA		
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA		
SUNDAY DAILY		50%	50%			
NUMBER OF STUDIES	11					
AVERAGE SIZE	485					
MINIMUM RATE	0.74	169	168	337		
AVERAGE RATE	1.29	294	293	587		
MAXIMUM RATE	2.28	519	518	1,037		
STANDARD DEVIATION	1.27					
EQUATION: LN (T) = 0.99 * LN(X) + 0.27	$R^2 = 0.86$	280	280	560		
LARGEST OF AVERAGE OR EQUATION		294	293	587		
PEAK HOUR GENERATOR		NA	NA			
NUMBER OF STUDIES	NA					
AVERAGE SIZE	NA					
MINIMUM RATE	NA	NA	NA	NA		
AVERAGE RATE	NA	NA	NA	NA		
MAXIMUM RATE	NA	NA	NA	NA		
STANDARD DEVIATION	NA					
EQUATION: NOT PROVIDED	NA	NA	NA	NA		
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA		
				EPS BROUP		

PROJECT		ANDERSON	RUSSELL	
		PARC		
ITE LAND USE CATEGORY AND CODE	SHOPPING CENTER - 820			
INDEPENDENT VARIABLE		1,000 SQU/	ARE FEET	
SIZE		90.1	69	
			TRIPS	
		ENTERING	EXITING	TOTAL
WEEKDAY DAILY		50%	50%	
NUMBER OF STUDIES	302			
AVERAGE SIZE	331			
MINIMUM RATE	12.50	564	563	1,127
AVERAGE RATE	42.70	1,925	1,925	3,850
MAXIMUM RATE	270.89	12,213	12,213	24,426
STANDARD DEVIATION	21.25			
EQUATION: LN (T) = 0.65 * LN(X) + 5.83	$R^2 = 0.79$	3,175	3,174	6,349
LARGEST OF AVERAGE OR EQUATION		3,175	3,174	6,349
AM PEAK HOUR ADJACENT STREET		62%	38%	
NUMBER OF STUDIES	104			
AVERAGE SIZE	310			
MINIMUM RATE	0.10	6	3	9
AVERAGE RATE	0.96	54	33	87
MAXIMUM RATE	9.05	506	310	816
STANDARD DEVIATION	1.31			
EQUATION: LN (T) = 0.61 * LN(X) + 2.24	$R^2 = 0.56$	91	55	146
LARGEST OF AVERAGE OR EQUATION		91	55	146
AM PEAK HOUR GENERATOR		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
PM PEAK HOUR ADJACENT STREET		48%	52%	
NUMBER OF STUDIES	426			
AVERAGE SIZE	376			
MINIMUM RATE	0.68	29	32	61
AVERAGE RATE	3.71	161	174	335
MAXIMUM RATE	29.27	1.267	1.372	2.639
STANDARD DEVIATION	2 74	.,	.,	_,000
EQUATION: $ N(T) = 0.67 * N(X) + 3.31$	$R^2 = 0.81$	268	291	559
LARGEST OF AVERAGE OR FOULTION	IX = 0.01	268	291	559
PM PEAK HOUR GENERATOR		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	ΝΔ	NA	NA	ΝΔ
	ΝΔ	NΔ	NΔ	NΔ
	ΝΔ		INA	
		ΝΔ	ΝΔ	ΝΔ
				NA NA
LANGEDI OF AVENAGE ON EQUATION	I	11/4	11/2	11/A
				CEPS GROUP

SIZL		50.	TDIDQ	
	RATE	ENTERING	EXITING	SLIM
SATURDAY DAILY		50%	50%	00111
NUMBER OF STUDIES	123	0070	0070	
AVERAGE SIZE	450			
MINIMUM RATE	16.70	753	753	1.506
AVERAGE RATE	49.97	2,253	2.253	4.506
MAXIMUM RATE	227.50	10.257	10.256	20.513
STANDARD DEVIATION	22.62			
EQUATION: LN (T) = 0.63 * LN(X) + 6.23	$R^2 = 0.82$	4.328	4.328	8.656
LARGEST OF AVERAGE OR EQUATION		4,328	4,328	8,656
PEAK HOUR GENERATOR		52%	48%	· · · · ·
NUMBER OF STUDIES	128			
AVERAGE SIZE	458			
MINIMUM RATE	1.46	69	63	132
AVERAGE RATE	4.82	226	209	435
MAXIMUM RATE	18.32	859	793	1,652
STANDARD DEVIATION	3.10			
EQUATION: LN (T) = 0.65 * LN(X) + 3.78	$R^2 = 0.83$	425	392	817
LARGEST OF AVERAGE OR EQUATION		425	392	817
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	439			
MINIMUM RATE	4.15	187	187	374
AVERAGE RATE	25.24	1,138	1,138	2,276
MAXIMUM RATE	148.15	6,680	6,679	13,359
STANDARD DEVIATION	17.23			
EQUATION: T = 15.63 * (X) + 4214.46	$R^2 = 0.52$	2,812	2,812	5,624
LARGEST OF AVERAGE OR EQUATION		2,812	2,812	5,624
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	39			
AVERAGE SIZE	369			
MINIMUM RATE	0.39	17	18	35
AVERAGE RATE	3.12	138	143	281
MAXIMUM RATE	12.40	548	570	1,118
STANDARD DEVIATION	2.78			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		138	143	281
				EPS CROUP

	SHOPPING CENTER - 820				
		1,000 SQUARE FEET			
SIZE	176.091				
				TOTAL	
		ENTERING	EXITING	TOTAL	
		50%	50%		
	302				
	331				
MINIMUM RATE	12.50	1,101	1,100	2,201	
AVERAGE RATE	42.70	3,760	3,759	7,519	
MAXIMUM RATE	270.89	23,851	23,850	47,701	
STANDARD DEVIATION	21.25				
EQUATION: LN (T) = 0.65 * LN(X) + 5.83	$R^2 = 0.79$	4,905	4,905	9,810	
LARGEST OF AVERAGE OR EQUATION		4,905	4,905	9,810	
AM PEAK HOUR ADJACENT STREET		62%	38%		
NUMBER OF STUDIES	104				
AVERAGE SIZE	310				
MINIMUM RATE	0.10	11	7	18	
AVERAGE RATE	0.96	105	64	169	
MAXIMUM RATE	9.05	988	606	1,594	
STANDARD DEVIATION	1.31				
EQUATION: LN (T) = 0.61 * LN(X) + 2.24	$R^2 = 0.56$	136	84	220	
LARGEST OF AVERAGE OR EQUATION		136	84	220	
AM PEAK HOUR GENERATOR		NA	NA		
NUMBER OF STUDIES	NA				
AVERAGE SIZE	NA				
MINIMUM RATE	NA	NA	NA	NA	
AVERAGE RATE	NA	NA	NA	NA	
MAXIMUM RATE	NA	NA	NA	NA	
STANDARD DEVIATION	NA				
EQUATION: NOT PROVIDED	NA	NA	NA	NA	
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA	
PM PEAK HOUR ADJACENT STREET		48%	52%		
NUMBER OF STUDIES	426				
AVERAGE SIZE	376				
	0.68	58	62	120	
AVERAGE RATE	3 71	313	.340	653	
	29.27	2 474	2 680	5 154	
STANDARD DEVIATION	20.21	<u> </u>	2,000	0,10-1	
EO(AT ON + 3.31)	$P^2 = 0.91$	420	455	875	
LARGEST OF AVERAGE OR FOLIATION	K = 0.01	420	455	875	
		<u>420</u> ΝΔ	<u>+35</u> ΝΔ	015	
	ΝΔ	11/1			
		ΝΛ	ΝΙΛ	NΙΛ	
		INA	INA	INA	
		NIA	NLA	NIA	
	NA	NA NA	NA NA	NA	
		NA	NA	NA	
				CEPS GROUP	

		1,000 300		
SIZL		170.	TDIDS	
	RATE	ENTERING	EXITING	SLIM
SATURDAY DAILY		50%	50%	00111
NUMBER OF STUDIES	123	0070	0070	
AVERAGE SIZE	450			
MINIMUM RATE	16.70	1.471	1.470	2.941
AVERAGE RATE	49.97	4,400	4,399	8,799
MAXIMUM RATE	227.50	20.031	20.030	40.061
STANDARD DEVIATION	22.62			
EQUATION: LN (T) = 0.63 * LN(X) + 6.23	$R^2 = 0.82$	6.599	6.598	13.197
LARGEST OF AVERAGE OR EQUATION	11 0102	6,599	6,598	13,197
PEAK HOUR GENERATOR		52%	48%	
NUMBER OF STUDIES	128			
AVERAGE SIZE	458			
MINIMUM RATE	1.46	134	123	257
AVERAGE RATE	4.82	441	408	849
MAXIMUM RATE	18.32	1,678	1,548	3,226
STANDARD DEVIATION	3.10			
EQUATION: LN (T) = 0.65 * LN(X) + 3.78	$R^2 = 0.83$	657	606	1,263
LARGEST OF AVERAGE OR EQUATION		657	606	1,263
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	439			
MINIMUM RATE	4.15	366	365	731
AVERAGE RATE	25.24	2,223	2,222	4,445
MAXIMUM RATE	148.15	13,044	13,044	26,088
STANDARD DEVIATION	17.23			
EQUATION: T = 15.63 * (X) + 4214.46	$R^2 = 0.52$	3,484	3,483	6,967
LARGEST OF AVERAGE OR EQUATION		3,484	3,483	6,967
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	39			
AVERAGE SIZE	369			
	0.39	34	35	69
AVERAGE RATE	3.12	269	280	549
MAXIMUM RATE	12.40	1,070	1,114	2,184
STANDARD DEVIATION	2.78			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		269	280	549
				CEPS GROUP

	ł	ANDERSON RUSSELL			
	l				
	APARTMENT - 220				
	l	DWELLING UNITS			
SIZE	 	45	<u>/8</u>	/	
	1 '		TRIPS	=	
	<u>بــــــــــــــــــــــــــــــــــــ</u>	ENTERING	EXITING	TOTAL	
WEEKDAY DAILY	<u>ا</u> '	50%	50%	ı	
NUMBER OF STUDIES	88	1	·	1 1	
AVERAGE SIZE	210	4		·	
MINIMUM RATE	1.27	291	291	582	
AVERAGE RATE	6.65	1,523	1,523	3,046	
MAXIMUM RATE	12.50	2,863	2,862	5,725	
STANDARD DEVIATION	3.07				
EQUATION: T = 6.06 * (X) + 123.56	$R^2 = 0.87$	1,450	1,449	2,899	
LARGEST OF AVERAGE OR EQUATION	[]	1,523	1,523	3,046	
AM PEAK HOUR ADJACENT STREET	'	20%	80%		
NUMBER OF STUDIES	78				
AVERAGE SIZE	235	1 1	·	1 I	
MINIMUM RATE	0.10	9	37	46	
AVFRAGE RATE	0.51	47	187	234	
MAXIMUM RATE	1.02	93	374	467	
STANDARD DEVIATION	0.73				
$F \cap I \Delta T \cap N \cdot T = 0.49 * (X) + 3.73$	$P^2 - 0.83$	46	182	228	
LARGEST OF AVERAGE OR EQUATION	Π = 0.00	47	187	234	
	<u>_</u>	29%	71%		
NUMBER OF STUDIES	83				
AV/ERAGE SIZE	230	1		1 I	
	0.10	13	22	46	
	0.10	73	170	252	
	1 0.55	144	251	195	
	0.76			400	
$ = 0.54 \times (Y) \pm 2.45 $	D ² 0.02	73	177	250	
	K = U.02	73	170	200	
	; <i>'</i>	13 650/	250/	232	
		0700	30%		
	90 000	1			
	200	20	16	46	
	0.10	3U 40E	01	40	
	0.62	100	99	284	
	1.64	488	263	/51	
STANDARD DEVIATION	0.82	170		070	
EQUATION: $1 = 0.55^{(X)} + 17.65$	$R^2 = 0.77$	1/6	94	270	
LARGEST OF AVERAGE OR EQUATION	<u> </u>	185	99	284	
PM PEAK HOUR GENERATOR	<u>ا</u> ــــــــــــــــــــــــــــــــــــ	61%	39%		
NUMBER OF STUDIES	85	4	·		
AVERAGE SIZE	229	I			
MINIMUM RATE	0.10	28	18	46	
AVERAGE RATE	0.67	187	120	307	
MAXIMUM RATE	1.64	458	293	751	
STANDARD DEVIATION	0.85	[]			
EQUATION: T = 0.60 * (X) + 14.91	$R^2 = 0.80$	177	113	290	
LARGEST OF AVERAGE OR EQUATION	'	187	120	307	
				EPS	
				GROUP	
1					

PROJECT		ANDERSON	RUSSELL	
PARCEL		PARC	ELG	
ITE LAND USE CATEGORY AND CODE	APARTMENT - 220			
	DWFLLING UNITS			
SIZE		45	8	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	15			
AVERAGE SIZE	175			
MINIMUM RATE	2.84	651	650	1,301
AVERAGE RATE	6.39	1,464	1,463	2,927
MAXIMUM RATE	8.40	1,924	1,923	3,847
STANDARD DEVIATION	2.99			
EQUATION: T = 7.85 * (X) - 256.19	$R^2 = 0.85$	1,670	1,669	3,339
LARGEST OF AVERAGE OR EQUATION		1,670	1,669	3,339
PEAK HOUR GENERATOR		50%	50%	
NUMBER OF STUDIES	14			
AVERAGE SIZE	178			
MINIMUM RATE	0.26	60	59	119
AVERAGE RATE	0.52	119	119	238
MAXIMUM RATE	1.05	241	240	481
STANDARD DEVIATION	0.74			
EQUATION: T = 0.41 * (X) + 19.23	$R^2 = 0.56$	104	103	207
LARGEST OF AVERAGE OR EQUATION		119	119	238
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	14			
AVERAGE SIZE	182			
MINIMUM RATE	3.21	735	735	1,470
AVERAGE RATE	5.86	1,342	1,342	2,684
MAXIMUM RATE	7.53	1,725	1,724	3,449
STANDARD DEVIATION	2.73			
EQUATION: T = 6.42 * (X) - 101.12	$R^2 = 0.82$	1,420	1,419	2,839
LARGEST OF AVERAGE OR EQUATION		1,420	1,419	2,839
PEAK HOUR GENERATOR		50%	50%	
NUMBER OF STUDIES	13			
AVERAGE SIZE	186			
MINIMUM RATE	0.26	60	59	119
AVERAGE RATE	0.51	117	117	234
MAXIMUM RATE	1.43	328	327	655
STANDARD DEVIATION	0.75			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		117	117	234
				EPS BROUP

	PARCEL H			
	RESIDENTIAL CONDOMINIUM / TOWNHOUSE - 230			
	DWELLING UNITS			
SIZE		04		
			EATTING	TOTAL
	50	50%	50%	
	50			
	179	05	64	100
	1.53	65	64	129
	5.81	244	244	488
	11.79	495	495	990
	3.11	077	070	
EQUATION: $LN(1) = 0.87 \ LN(X) + 2.46$	$R^2 = 0.80$	277	276	553
LARGEST OF AVERAGE OR EQUATION		277	276	553
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213			
MINIMUM RATE	0.15	2	11	13
AVERAGE RATE	0.44	6	31	37
MAXIMUM RATE	1.61	23	112	135
STANDARD DEVIATION	0.69			
EQUATION: LN (T) = 0.80 * LN(X) + 0.26	$R^2 = 0.76$	8	37	45
LARGEST OF AVERAGE OR EQUATION		8	37	45
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
MINIMUM RATE	0.15	2	11	13
AVERAGE RATE	0.44	7	30	37
MAXIMUM RATE	0.97	15	66	81
STANDARD DEVIATION	0.68			
EQUATION: LN (T) = 0.82 * LN(X) + 0.15	$R^2 = 0.80$	8	36	44
LARGEST OF AVERAGE OR EQUATION		8	36	44
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205			
MINIMUM RATE	0.18	10	5	15
AVERAGE RATE	0.52	29	15	44
MAXIMUM RATE	1.24	70	34	104
STANDARD DEVIATION	0.75			
EQUATION: LN (T) = 0.82 * LN(X) + 0.32	$R^2 = 0.80$	35	17	52
LARGEST OF AVERAGE OR EQUATION		35	17	52
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52			
AVERAGE SIZE	199			
MINIMUM RATE	0.18	10	5	15
AVERAGE RATE	0.52	28	16	44
MAXIMUM RATE	1.24	67	37	104
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	41	23	64
LARGEST OF AVERAGE OR EQUATION		41	23	64
				GROUP

PROJECT		ANDERSO		
PARCEL	PARCEL H			
ITE LAND USE CATEGORY AND CODE	RESIDENTIAL CONDOMINIUM / TOWNHOUSE - 230			
INDEPENDENT VARIABLE	DWELLING UNITS			
SIZE		8	4	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	49	49	98
AVERAGE RATE	5.67	238	238	476
MAXIMUM RATE	11.40	479	479	958
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	366	366	732
LARGEST OF AVERAGE OR EQUATION		366	366	732
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	6	6	12
AVERAGE RATE	0.47	21	18	39
MAXIMUM RATE	0.93	42	36	78
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	36	31	67
LARGEST OF AVERAGE OR EQUATION		36	31	67
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	57	57	114
AVERAGE RATE	4.84	204	203	407
MAXIMUM RATE	8.56	360	359	719
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	310	310	620
LARGEST OF AVERAGE OR EQUATION		310	310	620
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	6	7	13
AVERAGE RATE	0.45	19	19	38
MAXIMUM RATE	1.07	44	46	90
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	34	35	69
LARGEST OF AVERAGE OR EQUATION		34	35	69
				EPS BROUP

	RESIDENTIAL CONDOMINIUM / TOWNHOUSE - 230			
	DWELLING UNITS			
SIZE	133			
				TOTAL
		ENTERING	EXITING	TOTAL
	50	50%	50%	
	50			
	179	100	404	000
	1.53	102	101	203
	5.81	387	386	//3
	11.79	784	784	1,568
	3.11	440	110	00.4
EQUATION: $LN(1) = 0.87 \ LN(X) + 2.46$	$R^2 = 0.80$	412	412	824
LARGEST OF AVERAGE OR EQUATION		412	412	824
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213			
MINIMUM RATE	0.15	3	17	20
AVERAGE RATE	0.44	10	49	59
MAXIMUM RATE	1.61	36	178	214
STANDARD DEVIATION	0.69			
EQUATION: $LN(T) = 0.80 * LN(X) + 0.26$	$R^2 = 0.76$	11	54	65
LARGEST OF AVERAGE OR EQUATION		11	54	65
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
MINIMUM RATE	0.15	4	16	20
AVERAGE RATE	0.44	11	48	59
MAXIMUM RATE	0.97	25	104	129
STANDARD DEVIATION	0.68			
EQUATION: LN (T) = 0.82 * LN(X) + 0.15	$R^2 = 0.80$	12	52	64
LARGEST OF AVERAGE OR EQUATION		12	52	64
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205			
MINIMUM RATE	0.18	16	8	24
AVERAGE RATE	0.52	46	23	69
MAXIMUM RATE	1.24	111	54	165
STANDARD DEVIATION	0.75			
EQUATION: LN (T) = 0.82 * LN(X) + 0.32	$R^2 = 0.80$	51	25	76
LARGEST OF AVERAGE OR EQUATION		51	25	76
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52			
AVERAGE SIZE	199			
MINIMUM RATE	0.18	15	9	24
AVERAGE RATE	0.52	44	25	69
MAXIMUM RATE	1.24	106	59	165
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	52	29	81
LARGEST OF AVERAGE OR EQUATION		52	29	81
				GROUP
PROJECT	ANDERSON RUSSELL			
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PARCEL	PARCELI			
ITE LAND USE CATEGORY AND CODE	RESIDEN	TIAL CONDOMIN	IUM / TOWNHO	USE - 230
INDEPENDENT VARIABLE		DWELLIN	IG UNITS	
SIZE		13	33	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	78	78	156
AVERAGE RATE	5.67	377	377	754
MAXIMUM RATE	11.40	758	758	1,516
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	455	454	909
LARGEST OF AVERAGE OR EQUATION		455	454	909
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	10	9	19
AVERAGE RATE	0.47	34	29	63
MAXIMUM RATE	0.93	67	57	124
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	44	37	81
LARGEST OF AVERAGE OR EQUATION		44	37	81
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	91	90	181
AVERAGE RATE	4.84	322	322	644
MAXIMUM RATE	8.56	569	569	1,138
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	387	387	774
LARGEST OF AVERAGE OR EQUATION		387	387	774
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	10	11	21
AVERAGE RATE	0.45	29	31	60
MAXIMUM RATE	1.07	70	72	142
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	40	41	81
LARGEST OF AVERAGE OR EQUATION		40	41	81
				EPS GROUP

	DESIDEN			ISE - 220
	RESIDEN			J3E - 230
SIZE		04		
				τοται
			EXITING	TUTAL
	FC	50%	50%	
	00 470			
	179	<u></u>	<u> </u>	405
	1.53	03	02	120
	5.81	238	238	476
	11.79	484	483	967
	3.11	074	070	544
EQUATION: $LN(1) = 0.87 \ LN(X) + 2.46$	$R^2 = 0.80$	271	270	541
		2/1	270	541
	50	17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213		10	10
	0.15	2	10	12
AVERAGE RATE	0.44	6	30	36
	1.61	22	110	132
STANDARD DEVIATION	0.69			
EQUATION: $LN(T) = 0.80 * LN(X) + 0.26$	$R^2 = 0.76$	7	37	44
LARGEST OF AVERAGE OR EQUATION		7	37	44
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
	0.15	2	10	12
AVERAGE RATE	0.44	7	29	36
MAXIMUM RATE	0.97	15	65	80
STANDARD DEVIATION	0.68			
EQUATION: $LN(T) = 0.82 * LN(X) + 0.15$	$R^2 = 0.80$	8	35	43
LARGEST OF AVERAGE OR EQUATION		8	35	43
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205		_	
MINIMUM RATE	0.18	10	5	15
AVERAGE RATE	0.52	29	14	43
	1.24	68	34	102
STANDARD DEVIATION	0.75			
EQUATION: $LN(T) = 0.82 * LN(X) + 0.32$	$R^2 = 0.80$	34	17	51
LARGEST OF AVERAGE OR EQUATION		34	17	51
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52			
AVERAGE SIZE	199			
	0.18	10	5	15
AVERAGE RATE	0.52	28	15	43
MAXIMUM RATE	1.24	65	37	102
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	41	23	64
LARGEST OF AVERAGE OR EQUATION		41	23	64
				EPS
				GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL	PARCEL J			
ITE LAND USE CATEGORY AND CODE	RESIDEN	TIAL CONDOMIN	NUM / TOWNHO	USE - 230
INDEPENDENT VARIABLE		DWELLIN		
SIZE		8	2	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	48	48	96
AVERAGE RATE	5.67	233	232	465
MAXIMUM RATE	11.40	468	467	935
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	363	362	725
LARGEST OF AVERAGE OR EQUATION		363	362	725
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	6	5	11
AVERAGE RATE	0.47	21	18	39
MAXIMUM RATE	0.93	41	35	76
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	36	30	66
LARGEST OF AVERAGE OR EQUATION		36	30	66
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	56	56	112
AVERAGE RATE	4.84	199	198	397
MAXIMUM RATE	8.56	351	351	702
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	307	307	614
LARGEST OF AVERAGE OR EQUATION		307	307	614
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	6	7	13
AVERAGE RATE	0.45	18	19	37
MAXIMUM RATE	1.07	43	45	88
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	34	35	69
LARGEST OF AVERAGE OR EQUATION		34	35	69
				EPS GROUP

	DESIDEN			105 000
	KEƏIDEN			JSE - 230
			G UNITS	
SIZE		· · · · · · · · · · · · · · · · · · ·	2	
				TOTAL
				TUTAL
	50	50%	50%	
	56			
AVERAGE SIZE	1/9	50	50	110
	1.53	56	56	112
	5.81	212	212	424
	11.79	431	430	861
STANDARD DEVIATION	3.11			
EQUATION: LN (T) = $0.87 * LN(X) + 2.46$	$R^2 = 0.80$	245	244	489
LARGEST OF AVERAGE OR EQUATION		245	244	489
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59	1		
AVERAGE SIZE	213			
MINIMUM RATE	0.15	2	9	11
AVERAGE RATE	0.44	5	27	32
MAXIMUM RATE	1.61	20	98	118
STANDARD DEVIATION	0.69			
EQUATION: LN (T) = 0.80 * LN(X) + 0.26	$R^2 = 0.76$	7	33	40
LARGEST OF AVERAGE OR EQUATION		7	33	40
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196	1		
MINIMUM RATE	0.15	2	9	11
AVERAGE RATE	0.44	6	26	32
MAXIMUM RATE	0.97	13	58	71
STANDARD DEVIATION	0.68			
FOUATION: I N (T) = $0.82 \times LN(X) + 0.15$	$R^2 = 0.80$	7	32	39
I ARGEST OF AVERAGE OR EQUATION	10 - 0.00	7	32	39
PM PEAK HOUR ADJACENT STREET		67%	33%	
	62	01,0	0070	
	205	1		
	0.18	q	4	13
	0.10	25	12	28
	1.04	2J 61	30	<u>01</u>
	0.75	01	30	91
	$D^2 0.00$	21	15	46
EQUATION: LIN (1) = 0.02 LIN(A) + 0.32	R ⁻ = 0.80	31 24	G	40
	·	5 1	15	40
	50	04%	30%	
	⊃∠ 400	1		
	199		F	40
	0.18	8	5	13
	0.52	24	14	38
	1.24	58	33	91
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	39	22	61
LARGEST OF AVERAGE OR EQUATION		39	22	61
				C EPS
				SROUP GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL	PARCEL K			
ITE LAND USE CATEGORY AND CODE	RESIDEN	TIAL CONDOMIN	IUM / TOWNHOU	USE - 230
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		7:	3	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	43	42	85
AVERAGE RATE	5.67	207	207	414
MAXIMUM RATE	11.40	416	416	832
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	346	346	692
LARGEST OF AVERAGE OR EQUATION		346	346	692
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	5	5	10
AVERAGE RATE	0.47	18	16	34
MAXIMUM RATE	0.93	37	31	68
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	35	29	64
LARGEST OF AVERAGE OR EQUATION		35	29	64
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	50	49	99
AVERAGE RATE	4.84	177	176	353
MAXIMUM RATE	8.56	313	312	625
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	293	293	586
LARGEST OF AVERAGE OR EQUATION		293	293	586
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	6	6	12
AVERAGE RATE	0.45	16	17	33
MAXIMUM RATE	1.07	38	40	78
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	33	34	67
LARGEST OF AVERAGE OR EQUATION		33	34	67
				EPS BROUP

	l			
	l			
	l			
512E	i	30	<u>\$</u>	
	1			TOTAL
	j	ENTERING	EXTTING	TUTAL
	255	50%	50%	
	355	i 1		
AVERAGE SIZE	198		105	
MINIMUM RATE	4.31	125	125	250
AVERAGE RATE	9.52	276	276	552
MAXIMUM RATE	21.85	634	633	1,267
STANDARD DEVIATION	3.70	l		
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	318	318	636
LARGEST OF AVERAGE OR EQUATION		318	318	636
AM PEAK HOUR ADJACENT STREET		25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194	l		
MINIMUM RATE	0.33	5	14	19
AVERAGE RATE	0.75	11	33	44
MAXIMUM RATE	2.27	33	99	132
STANDARD DEVIATION	0.90	l I		
EQUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	13	37	50
LARGEST OF AVERAGE OR EQUATION	[]	13	37	50
AM PEAK HOUR GENERATOR		26%	74%	
NUMBER OF STUDIES	343	1		
AVERAGE SIZE	180	1		
MINIMUM RATE	0.33	5	14	19
AVFRAGE RATE	0.77	12	33	45
MAXIMUM RATE	2.27	34	98	132
STANDARD DEVIATION	0.91			
FOUNTION: $T = 0.70 * (X) + 12.12$	$P^2 = 0.89$	14	39	53
LARGEST OF AVERAGE OR EQUATION	<u> </u>	14	39	53
PM PFAK HOUR ADJACENT STREET	ii	63%	37%	
NUMBER OF STUDIES	321	0070	0170	
AV/FRAGE SIZE	207	i 1	I	
	0.42	15	Q	24
	1 00	37	21	<u>-</u> 58
	2 08	100	64	172
	1.05	103	U-T	175
$= 0.00 \times 100 \times 1000 \times 1000$	$D^2 = 0.01$	40	24	61
LADOEST OF AVERAGE OR FOLIATION	K = 0.91	40	24 24	64 64
	; [!]	40 61%	24 260/	<u>U4</u>
	362	04 /0	3070	
		i 1	I	
	0.42	15	0	24
	0.42 4 02	10	ح 01	24 50
	1.02	30 444	21	59 470
	2.90	111	<u></u> 6∠	1/3
	1.05	40		00
EQUATION: LN (1) = $0.88 \text{ LN}(\lambda) + 0.02$	R ⁻ = 0.91	42	24	66
LARGEST OF AVERAGE OK EQUATION	<u> </u>	42	24	66
				EPS BROUP

PROJECT				
PARCEL		PARC	ELL	
ITE LAND USE CATEGORY AND CODE	SINGLE FAMILY - 210			
		DWELLIN		
SIZE		5	8	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	155	154	309
AVERAGE RATE	9.91	288	287	575
MAXIMUM RATE	15.25	443	442	885
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	306	306	612
LARGEST OF AVERAGE OR EQUATION		306	306	612
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	16	13	29
AVERAGE RATE	0.93	29	25	54
MAXIMUM RATE	1.75	55	47	102
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	32	28	60
LARGEST OF AVERAGE OR EQUATION		32	28	60
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	138	137	275
AVERAGE RATE	8.62	250	250	500
MAXIMUM RATE	12.31	357	357	714
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	250	250	500
LARGEST OF AVERAGE OR EQUATION		250	250	500
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	17	15	32
AVERAGE RATE	0.86	27	23	50
MAXIMUM RATE	1.48	46	40	86
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	29	26	55
LARGEST OF AVERAGE OR EQUATION		29	26	55
				C EPS

	DESIDEN			ISE 220
	RESIDEN			J3E - 230
		DWELLIN		
SIZE		50		
				ΤΟΤΑΙ
		ENTERING	EXITING	TOTAL
	50	50%	50%	
	50 470			
	179	20	00	77
	1.53	39	38	11
	5.81	146	145	291
	11.79	295	295	590
	3.11	470	170	050
EQUATION: $LN(1) = 0.87 \ LN(X) + 2.46$	$R^2 = 0.80$	176	176	352
		1/6	176	352
	50	17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213		_	
	0.15	1	7	8
	0.44	4	18	22
	1.61	14	67	81
STANDARD DEVIATION	0.69			
EQUATION: $LN(T) = 0.80 * LN(X) + 0.26$	$R^2 = 0.76$	5	25	30
LARGEST OF AVERAGE OR EQUATION		5	25	30
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
MINIMUM RATE	0.15	2	6	8
AVERAGE RATE	0.44	4	18	22
MAXIMUM RATE	0.97	9	40	49
STANDARD DEVIATION	0.68			
EQUATION: LN (T) = 0.82 * LN(X) + 0.15	$R^2 = 0.80$	6	23	29
LARGEST OF AVERAGE OR EQUATION		6	23	29
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205			
MINIMUM RATE	0.18	6	3	9
AVERAGE RATE	0.52	17	9	26
MAXIMUM RATE	1.24	42	20	62
STANDARD DEVIATION	0.75			
EQUATION: LN (T) = 0.82 * LN(X) + 0.32	$R^2 = 0.80$	23	11	34
LARGEST OF AVERAGE OR EQUATION		23	11	34
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52			
AVERAGE SIZE	199			
MINIMUM RATE	0.18	6	3	9
AVERAGE RATE	0.52	17	9	26
MAXIMUM RATE	1.24	40	22	62
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	34	19	53
LARGEST OF AVERAGE OR EQUATION		34	19	53
				FPS
				GROUP

PROJECT		ANDERSON		
PARCEL		PARC	EL N	
ITE LAND USE CATEGORY AND CODE	RESIDENTIAL CONDOMINIUM / TOWNHOUSE - 230			USE - 230
INDEPENDENT VARIABLE		DWELLIN		
SIZE		5	0	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	30	29	59
AVERAGE RATE	5.67	142	142	284
MAXIMUM RATE	11.40	285	285	570
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	305	304	609
LARGEST OF AVERAGE OR EQUATION		305	304	609
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	4	3	7
AVERAGE RATE	0.47	13	11	24
MAXIMUM RATE	0.93	25	22	47
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	31	26	57
LARGEST OF AVERAGE OR EQUATION		31	26	57
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	34	34	68
AVERAGE RATE	4.84	121	121	242
MAXIMUM RATE	8.56	214	214	428
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	257	257	514
LARGEST OF AVERAGE OR EQUATION		257	257	514
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
	0.16	4	4	8
AVERAGE RATE	0.45	11	12	23
	1.07	26	28	54
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	30	32	62
LARGEST OF AVERAGE OR EQUATION		30	32	62
				EPS

	PESIDEN			ISE - 230
	RESIDEN			J3E - 230
SIZE		94		
				ΤΟΤΑΙ
			EATTING	TUTAL
	FC	50%	50%	
	30			
	179	74	70	4.4.4
	1.53	71	70	141
	5.81	268	267	535
	11.79	543	542	1,085
STANDARD DEVIATION	3.11			
EQUATION: $LN(1) = 0.87 * LN(X) + 2.46$	$R^2 = 0.80$	299	299	598
LARGEST OF AVERAGE OR EQUATION		299	299	598
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213			
MINIMUM RATE	0.15	2	12	14
AVERAGE RATE	0.44	7	33	40
MAXIMUM RATE	1.61	25	123	148
STANDARD DEVIATION	0.69			
EQUATION: LN (T) = 0.80 * LN(X) + 0.26	$R^2 = 0.76$	8	40	48
LARGEST OF AVERAGE OR EQUATION		8	40	48
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
MINIMUM RATE	0.15	3	11	14
AVERAGE RATE	0.44	8	32	40
MAXIMUM RATE	0.97	17	72	89
STANDARD DEVIATION	0.68			
EQUATION: LN (T) = 0.82 * LN(X) + 0.15	$R^2 = 0.80$	9	38	47
LARGEST OF AVERAGE OR EQUATION		9	38	47
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205			
MINIMUM RATE	0.18	11	6	17
AVERAGE RATE	0.52	32	16	48
MAXIMUM RATE	1.24	76	38	114
STANDARD DEVIATION	0.75			
EQUATION: LN (T) = 0.82 * LN(X) + 0.32	$R^2 = 0.80$	38	18	56
LARGEST OF AVERAGE OR EQUATION		38	18	56
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52			
AVERAGE SIZE	199			
MINIMUM RATE	0.18	11	6	17
AVERAGE RATE	0.52	31	17	48
MAXIMUM RATE	1.24	73	41	114
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	43	24	67
LARGEST OF AVERAGE OR EQUATION		43	24	67
				GROUP

PROJECT	ANDERSON RUSSELL				
PARCEL	PARCEL O				
ITE LAND USE CATEGORY AND CODE	RESIDEN	ITIAL CONDOMIN	IUM / TOWNHO	USE - 230	
INDEPENDENT VARIABLE		DWELLIN	IG UNITS		
SIZE		9	2		
			TRIPS		
	RATE	ENTERING	EXITING	SUM	
SATURDAY DAILY		50%	50%		
NUMBER OF STUDIES	30				
AVERAGE SIZE	209				
MINIMUM RATE	1.17	54	54	108	
AVERAGE RATE	5.67	261	261	522	
MAXIMUM RATE	11.40	525	524	1,049	
STANDARD DEVIATION	3.10				
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	381	380	761	
LARGEST OF AVERAGE OR EQUATION		381	380	761	
PEAK HOUR GENERATOR		54%	46%		
NUMBER OF STUDIES	27				
AVERAGE SIZE	228				
MINIMUM RATE	0.14	7	6	13	
AVERAGE RATE	0.47	23	20	43	
MAXIMUM RATE	0.93	46	40	86	
STANDARD DEVIATION	0.71				
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	37	32	69	
LARGEST OF AVERAGE OR EQUATION		37	32	69	
SUNDAY DAILY		50%	50%		
NUMBER OF STUDIES	30				
AVERAGE SIZE	209				
MINIMUM RATE	1.36	63	62	125	
AVERAGE RATE	4.84	223	222	445	
MAXIMUM RATE	8.56	394	394	788	
STANDARD DEVIATION	2.71				
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	323	322	645	
LARGEST OF AVERAGE OR EQUATION		323	322	645	
PEAK HOUR GENERATOR		49%	51%		
NUMBER OF STUDIES	27				
AVERAGE SIZE	228				
MINIMUM RATE	0.16	7	8	15	
AVERAGE RATE	0.45	20	21	41	
MAXIMUM RATE	1.07	48	50	98	
STANDARD DEVIATION	0.70				
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	35	36	71	
LARGEST OF AVERAGE OR EQUATION		35	36	71	
				CEPS BROUP	

	i		<u>, EL F</u> MII V - 210	
	<u> </u>			
	<u> </u>	7		
SIZE	i	r		
	1 7	ENTERING		τοται
	i	ENTERING 50%	EATTING 50%	
	255	5076	5076	I
	108	1 1	i – 🖡	I T
AVERAGE SIZE	130	162	161	272
	4.51	257	257	323 711
	9.02 01.85	820	207 910	1 630
	21.00	020	619	قەتى, I
STANDARD DEVIATION $\Box O U A T O N U N (T) = 0.02 * 1 N(X) + 2.72$	5.70 5 ² 0.05	402	402	906
EQUATION. LIV $(1) = 0.32$ LIV $(\Lambda) + 2.72$	R ⁻ = 0.95	403	403	000
LAKGEST OF AVERAGE ON LOUGHING	<u></u>	403 250/	403 750/	000
AM PEAK HOUK ADJAGENT STREET	202	2370	/5%	I
	292	1 1	i	1 1
	194		40	<u> </u>
	0.33	0	19	20
	0.75	14	42	0C
	2.27	43	127	170
STANDARD DEVIATION	0.90			
EQUATION: $I = 0.70^{\circ}(X) + 9.74$	R [∠] = 0.89	16	46	62
	<u> </u>	16	46	62
AM PEAK HOUR GENERATUR		26%	74%	I
NUMBER OF STUDIES	343	1 1	i – 🖡	1
AVERAGE SIZE	180	↓		~
MINIMUM RATE	0.33	7	18	25
AVERAGE RATE	0.77	15	43	58
MAXIMUM RATE	2.27	44	126	170
STANDARD DEVIATION	0.91	L	·	
EQUATION: T = 0.70 * (X) + 12.12	$R^2 = 0.89$	17	48	65
LARGEST OF AVERAGE OR EQUATION	<u> </u>	17	48	65
PM PEAK HOUR ADJACENT STREET	[!	63%	37%	
NUMBER OF STUDIES	321	1	i – 🖡	
AVERAGE SIZE	207		·	
MINIMUM RATE	0.42	20	12	32
AVERAGE RATE	1.00	47	28	75
MAXIMUM RATE	2.98	141	83	224
STANDARD DEVIATION	1.05		·]	
EQUATION: LN (T) = 0.90 * LN(X) + 0.51	$R^2 = 0.91$	51	30	81
LARGEST OF AVERAGE OR EQUATION	!	51	30	81
PM PEAK HOUR GENERATOR	!	64%	36%	
NUMBER OF STUDIES	362	1 1	i	1
AVERAGE SIZE	174	l	L	1
MINIMUM RATE	0.42	20	12	32
AVERAGE RATE	1.02	49	28	77
MAXIMUM RATE	2.98	143	81	224
STANDARD DEVIATION	1.05		 //	
EQUATION: LN (T) = 0.88 * LN(X) + 0.62	$R^2 = 0.91$	53	30	83
LARGEST OF AVERAGE OR EQUATION	I!	53	30	83
				EPS
				GROUP
4				

PROJECT				
PARCEI				
ITE LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
		DWELLIN		
SIZE		7	5	
		-	TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	200	199	399
AVERAGE RATE	9.91	372	371	743
MAXIMUM RATE	15.25	572	572	1,144
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	389	388	777
LARGEST OF AVERAGE OR EQUATION		389	388	777
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	21	17	38
AVERAGE RATE	0.93	38	32	70
MAXIMUM RATE	1.75	71	60	131
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	41	35	76
LARGEST OF AVERAGE OR EQUATION		41	35	76
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	178	178	356
AVERAGE RATE	8.62	324	323	647
MAXIMUM RATE	12.31	462	461	923
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	324	323	647
LARGEST OF AVERAGE OR EQUATION		324	323	647
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	22	19	41
AVERAGE RATE	0.86	34	31	65
MAXIMUM RATE	1.48	59	52	111
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	37	32	69
LARGEST OF AVERAGE OR EQUATION		37	32	69
				C EPS

	DESIDEN			ISE - 220
	RESIDEN			J3E - 230
SIZE		10		
				TOTAL
		ENTERING	EXITING	TOTAL
	50	50%	50%	
	50			
	179	70	77	455
	1.53	78	11	155
	5.81	294	293	587
	11.79	596	595	1,191
	3.11	005	004	0.40
EQUATION: $LN(1) = 0.87 \ LN(X) + 2.46$	$R^2 = 0.80$	325	324	649
LARGEST OF AVERAGE OR EQUATION		325	324	649
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213			
MINIMUM RATE	0.15	3	12	15
AVERAGE RATE	0.44	7	37	44
MAXIMUM RATE	1.61	28	135	163
STANDARD DEVIATION	0.69			
EQUATION: $LN(T) = 0.80 * LN(X) + 0.26$	$R^2 = 0.76$	9	43	52
LARGEST OF AVERAGE OR EQUATION		9	43	52
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
MINIMUM RATE	0.15	3	12	15
AVERAGE RATE	0.44	8	36	44
MAXIMUM RATE	0.97	19	79	98
STANDARD DEVIATION	0.68			
EQUATION: LN (T) = 0.82 * LN(X) + 0.15	$R^2 = 0.80$	10	41	51
LARGEST OF AVERAGE OR EQUATION		10	41	51
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205			
MINIMUM RATE	0.18	12	6	18
AVERAGE RATE	0.52	36	17	53
MAXIMUM RATE	1.24	84	41	125
STANDARD DEVIATION	0.75			
EQUATION: LN (T) = 0.82 * LN(X) + 0.32	$R^2 = 0.80$	41	20	61
LARGEST OF AVERAGE OR EQUATION		41	20	61
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52			
AVERAGE SIZE	199			
MINIMUM RATE	0.18	12	6	18
AVERAGE RATE	0.52	34	19	53
MAXIMUM RATE	1.24	80	45	125
STANDARD DEVIATION	0.75			
EQUATION: T = 0.34 * (X) + 35.87	$R^2 = 0.82$	45	25	70
LARGEST OF AVERAGE OR EQUATION		45	25	70
				GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL	PARCEL Q			
ITE LAND USE CATEGORY AND CODE	RESIDEN	ITIAL CONDOMIN	IUM / TOWNHO	USE - 230
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		10	1	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	59	59	118
AVERAGE RATE	5.67	287	286	573
MAXIMUM RATE	11.40	576	575	1,151
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	397	397	794
LARGEST OF AVERAGE OR EQUATION		397	397	794
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	8	6	14
AVERAGE RATE	0.47	25	22	47
MAXIMUM RATE	0.93	51	43	94
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	39	33	72
LARGEST OF AVERAGE OR EQUATION		39	33	72
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	69	68	137
AVERAGE RATE	4.84	245	244	489
MAXIMUM RATE	8.56	433	432	865
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	337	336	673
LARGEST OF AVERAGE OR EQUATION		337	336	673
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	8	8	16
AVERAGE RATE	0.45	22	23	45
MAXIMUM RATE	1.07	53	55	108
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	36	37	73
LARGEST OF AVERAGE OR EQUATION		36	37	73
				EPS GROUP

PKUJECI			RUSSELL	
		PAKU		
	RESIDEN	TIAL CONDOMIN	IUM / TOWNHOU	JSE - 230
		DWELLIN	G UNITS	
SIZE	 	/:	5	
			I RIPS	70741
		ENTERING	EXITING	TOTAL
		50%	50%	
NUMBER OF STUDIES	56	l		
AVERAGE SIZE	179			· · -
MINIMUM RATE	1.53	58	57	115
AVERAGE RATE	5.81	218	218	436
MAXIMUM RATE	11.79	442	442	884
STANDARD DEVIATION	3.11			
EQUATION: LN (T) = 0.87 * LN(X) + 2.46	$R^2 = 0.80$	251	250	501
LARGEST OF AVERAGE OR EQUATION		251	250	501
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213			
MINIMUM RATE	0.15	2	9	11
AVERAGE RATE	0.44	6	27	33
MAXIMUM RATE	1.61	21	100	121
STANDARD DEVIATION	0.69			
EQUATION: LN (T) = 0.80 * LN(X) + 0.26	$R^2 = 0.76$	7	34	41
LARGEST OF AVERAGE OR EQUATION		7	34	41
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVFRAGE SIZE	196	1		
MINIMUM RATE	0.15	2	9	11
AVFRAGE RATE	0.44	6	27	33
MAXIMUM RATE	0.97	14	59	73
STANDARD DEVIATION	0.68	l · · ·		
FOI IATION' I N (T) = 0.82 * LN(X) + 0.15	$P^2 = 0.80$	8	32	40
LARGEST OF AVERAGE OR EQUATION	IX = 0.00	8	32	40
PM PFAK HOUR ADJACENT STREET	;;	67%	33%	
NUMBER OF STUDIES	62	01,0		
	205	1		
	0.18	Q	5	14
	0.10	26	13	20
	1.04	20 62	21	03
	0.75	02	51	30
	0.73	21	16	17
EQUATION. LIN (1) = 0.02 LIN(A) ± 0.02	R⁻ = ∪.ŏ∪	় 3 1	10	41 A7
	<u> </u> 	3 1		41
	E 2	0470	30%	
	52 100	1		
	199		E	4.4
	0.10	9	C A A	14
	0.52	25	14	<u> </u>
	1.24	60	33	93
	0.75			<u></u>
EQUATION: $T = 0.34 * (X) + 35.87$	$R^2 = 0.82$	39	22	61
LARGEST OF AVERAGE OR EQUATION		39	22	61
				EPS
				GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL		PARC	ELR	
ITE LAND USE CATEGORY AND CODE	RESIDEN	ITIAL CONDOMIN		JSE - 230
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		7	5	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	44	44	88
AVERAGE RATE	5.67	213	212	425
MAXIMUM RATE	11.40	428	427	855
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	350	349	699
LARGEST OF AVERAGE OR EQUATION		350	349	699
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	6	5	11
AVERAGE RATE	0.47	19	16	35
MAXIMUM RATE	0.93	38	32	70
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	35	29	64
LARGEST OF AVERAGE OR EQUATION		35	29	64
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	51	51	102
AVERAGE RATE	4.84	182	181	363
MAXIMUM RATE	8.56	321	321	642
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	296	296	592
LARGEST OF AVERAGE OR EQUATION		296	296	592
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	6	6	12
AVERAGE RATE	0.45	17	17	34
MAXIMUM RATE	1.07	39	41	80
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	33	34	67
LARGEST OF AVERAGE OR EQUATION		33	34	67
				EPS BROUP

		ANDERSON	RUSSELL	
			EL S	
		SINGLE FAI	MILY - 210	
			GUNITS	
SIZE		13	3	
			TRIPS	
		ENTERING	EXITING	TOTAL
WEEKDAY DAILY		50%	50%	
NUMBER OF STUDIES	355			
AVERAGE SIZE	198			
MINIMUM RATE	4.31	287	286	573
AVERAGE RATE	9.52	633	633	1,266
MAXIMUM RATE	21.85	1,453	1,453	2,906
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	683	682	1,365
LARGEST OF AVERAGE OR EQUATION		683	682	1,365
AM PEAK HOUR ADJACENT STREET		25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194			
MINIMUM RATE	0.33	11	33	44
AVERAGE RATE	0.75	25	75	100
MAXIMUM RATE	2.27	76	226	302
STANDARD DEVIATION	0.90		i	
FOUNTION: $T = 0.70 * (X) + 9.74$	$R^2 = 0.89$	26	77	103
I ARGEST OF AVERAGE OR EQUATION	<u> </u>	26	77	103
AM PEAK HOUR GENERATOR	I	26%	74%	
NUMBER OF STUDIES	343			
AVFRAGE SIZE	180		I	
MINIMUM RATE	0.33	11	33	44
AVFRAGE RATE	0.77	27	75	102
MAXIMUM RATE	2.27	79	223	302
STANDARD DEVIATION	0.91			
$F \cap I \Delta T \cap N! T = 0.70 * (X) + 12.12$	$P^2 = 0.89$	27	78	105
APGEST OF AVERAGE OR EQUATION	n = 0.00	27	78	105
PM PFAK HOUR AD.IACENT STREET		63%	37%	
	321	0070	0170	
	2∩7			
	0.42	25	21	56
	1.00	93 94	<u>ک</u> ا ۸۵	122
	1.00 2 QQ	2/0	40	206
	2.30	243	147	390
	5^{2} 0.01	20	50	126
EQUATION. LIN $(1) = 0.50$ LIN $(\Lambda) = 0.51$	K⁻ = 0.91	00 20	50	130
	 A	0 0	JU	130
	260	0470	30%	
	<u> </u>		I	
	0.42	26	20	FC
	0.42	30	20	00
	1.02	87	49	130
	2.98	253	143	396
	1.05			407
EQUATION: LN (1) = $0.88 \text{ LN}(\lambda) + 0.62$	$R^2 = 0.91$	88	49	137
		88	49	137
				EPS
				W 6KUUP

PROJECT		ANDERSON		
PARCEI		PARC	ELS	
ITE LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
		DWELLIN		
SIZE		13	3	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	354	354	708
AVERAGE RATE	9.91	659	659	1,318
MAXIMUM RATE	15.25	1,014	1,014	2,028
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	662	661	1,323
LARGEST OF AVERAGE OR EQUATION		662	661	1,323
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	36	31	67
AVERAGE RATE	0.93	67	57	124
MAXIMUM RATE	1.75	126	107	233
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	69	58	127
LARGEST OF AVERAGE OR EQUATION		69	58	127
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	315	315	630
AVERAGE RATE	8.62	573	573	1,146
MAXIMUM RATE	12.31	819	818	1,637
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	574	573	1,147
LARGEST OF AVERAGE OR EQUATION		574	573	1,147
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	39	34	73
AVERAGE RATE	0.86	60	54	114
MAXIMUM RATE	1.48	104	93	197
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	62	55	117
LARGEST OF AVERAGE OR EQUATION		62	55	117
				EPS BROUP

n				
PROJECI	l			
	l		EL T	
ITE LAND USE CATEGORY AND CODE	l	ELEMENTARY	SCHOOL - 520	
	ļ	STUD	ENTS	
SIZE	<u> </u>	600.	000	
l I	1 '		TRIPS	
I	<u> </u>	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	ļ'	50%	50%	ıĮ
NUMBER OF STUDIES	33	1	. J	1 1
AVERAGE SIZE	620			·
MINIMUM RATE	0.45	135	135	270
AVERAGE RATE	1.29	387	387	774
MAXIMUM RATE	2.45	735	735	1,470
STANDARD DEVIATION	1.26			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION	['	387	387	774
AM PEAK HOUR ADJACENT STREET	'	NA	NA	
NUMBER OF STUDIES	NA		,,	
AVERAGE SIZE	NA	1	. J	1 1
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA	h	T	· ·
	NA	NA	NA	NA
ARGEST OF AVERAGE OR EQUATION	·····	NA	NA	NA
	·	55%	45%	
NUMBER OF STUDIES	48		-1070	
	630	1 1	. J	1 I
	0.00	36	30	66
	0.11	1/0	101	270
	0.45	204	121 2/1Q	210 552
	0.92	304	240	ວບ∠
$\frac{\text{STANDARD DEVIATION}}{\text{STANDARD DEVIATION}} = 1.14 \times 1.0(Y) = 1.86$	-0.70	106	402	220
EQUATION: LIN $(1) = 1.14$ LIN $(\Lambda) = 1.00$	$R^2 = 0.50$	120	103	229
LARGEST OF AVERAGE OK EQUATION	<u></u> /	149		270
PM PEAK HOUR ADJACENT STREET	└─ <u>─</u>	49%	51%	
NUMBER OF STUDIES	20	i	. J	
AVERAGE SIZE	18/	↓	·	
MINIMUM RATE	0.05	15	15	30
AVERAGE RATE	0.15	44	46	90
MAXIMUM RATE	0.37	109	113	222
STANDARD DEVIATION	0.40	l		
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION	'	44	46	90
PM PEAK HOUR GENERATOR	 '	45%	55%	
NUMBER OF STUDIES	44		. I	
AVERAGE SIZE	642		·	
MINIMUM RATE	0.09	24	30	54
AVERAGE RATE	0.28	76	92	168
	0.50	135	165	300
STANDARD DEVIATION	0.54			
EQUATION: LN (T) = 1.09 * <u>LN(X) - 1.92</u>	$R^2 = 0.54$	194	236	430
LARGEST OF AVERAGE OR EQUATION	ſ <u>, , , , , , , , , , , , , , , , , , , </u>	194	236	430
				FPS
				GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL		PARC	ELT	
ITE LAND USE CATEGORY AND CODE		ELEMENTARY	 SCHOOL - 520	
		STUDI		
SIZE		600.	000	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
PEAK HOUR GENERATOR		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
SUNDAY DAILY		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
PEAK HOUR GENERATOR		NA	NA	
NUMBER OF STUDIES	NA			
AVERAGE SIZE	NA			
MINIMUM RATE	NA	NA	NA	NA
AVERAGE RATE	NA	NA	NA	NA
MAXIMUM RATE	NA	NA	NA	NA
STANDARD DEVIATION	NA			
EQUATION: NOT PROVIDED	NA	NA	NA	NA
LARGEST OF AVERAGE OR EQUATION		NA	NA	NA
				EPS BROUP

	DESIDEN			ICE 220
	REJIDEN			JSE - 230
SIZE		30	2	
				τοται
				TUTAL
	EC	50%	50%	
	00			
	1/9	75	75	450
	1.53	/5	/5	150
	5.81	285	284	569
	11.79	578	577	1,155
STANDARD DEVIATION	3.11			
EQUATION: LN (T) = $0.87 \times LN(X) + 2.46$	$R^2 = 0.80$	316	316	632
LARGEST OF AVERAGE OR EQUATION		316	316	632
AM PEAK HOUR ADJACENT STREET		17%	83%	
NUMBER OF STUDIES	59			
AVERAGE SIZE	213	l		
MINIMUM RATE	0.15	3	12	15
AVERAGE RATE	0.44	7	36	43
MAXIMUM RATE	1.61	27	131	158
STANDARD DEVIATION	0.69			
EQUATION: LN (T) = 0.80 * LN(X) + 0.26	$R^2 = 0.76$	9	42	51
LARGEST OF AVERAGE OR EQUATION		9	42	51
AM PEAK HOUR GENERATOR		19%	81%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	196			
MINIMUM RATE	0.15	3	12	15
AVERAGE RATE	0.44	8	35	43
MAXIMUM RATE	0.97	18	77	95
STANDARD DEVIATION	0.68			
EQUATION: LN (T) = 0.82 * LN(X) + 0.15	$R^2 = 0.80$	10	40	50
LARGEST OF AVERAGE OR EQUATION		10	40	_50
PM PEAK HOUR ADJACENT STREET		67%	33%	
NUMBER OF STUDIES	62			
AVERAGE SIZE	205			
MINIMUM RATE	0.18	12	6	18
AVERAGE RATE	0.52	34	17	51
MAXIMUM RATE	1.24	82	40	122
STANDARD DEVIATION	0.75		-	
FOUATION: LN (T) = $0.82 \times LN(X) + 0.32$	$R^2 = 0.80$	40	19	59
I ARGEST OF AVERAGE OR EQUATION	10 - 0.00	40	19	59
PM PFAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	52	01/0	0070	
AVFRAGE SIZE	199			
	0.18	12	6	18
	0.52	33	18	51
	1 24	78	44	122
ΥΤΔΝΙΔΩΩ ΠΕ\/ΙΔΤΙΩΝΙ	0.75	10		122
$= 0.114 \pm 0.011 \pm 0.024 \pm 0.011 \pm 0.0011 \pm 0.00111 \pm 0.0011 \pm 0.001100011 \pm 0.00110000000000$	$D^2 \Delta Q^2$	11	25	60
EQUATION: $I = 0.34$ (A) ± 33.07	K = U.o∠	44	20 25	60
LAKGEST OF AVERAGE ON EQUATION		44	29	03
				C EPS

PROJECT	ANDERSON RUSSELL			
PARCEL	PARCEL U			
ITE LAND USE CATEGORY AND CODE	RESIDEN	ITIAL CONDOMIN		USE - 230
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		98	8	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.17	58	57	115
AVERAGE RATE	5.67	278	278	556
MAXIMUM RATE	11.40	559	558	1,117
STANDARD DEVIATION	3.10			
EQUATION: T = 3.62 * (X) + 427.93	$R^2 = 0.84$	392	391	783
LARGEST OF AVERAGE OR EQUATION		392	391	783
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.14	8	6	14
AVERAGE RATE	0.47	25	21	46
MAXIMUM RATE	0.93	49	42	91
STANDARD DEVIATION	0.71			
EQUATION: T = 0.29 * (X) + 42.63	$R^2 = 0.84$	38	33	71
LARGEST OF AVERAGE OR EQUATION		38	33	71
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	30			
AVERAGE SIZE	209			
MINIMUM RATE	1.36	67	66	133
AVERAGE RATE	4.84	237	237	474
MAXIMUM RATE	8.56	420	419	839
STANDARD DEVIATION	2.71			
EQUATION: T = 3.13 * (X) + 357.26	$R^2 = 0.88$	332	332	664
LARGEST OF AVERAGE OR EQUATION		332	332	664
PEAK HOUR GENERATOR		49%	51%	
NUMBER OF STUDIES	27			
AVERAGE SIZE	228			
MINIMUM RATE	0.16	8	8	16
AVERAGE RATE	0.45	22	22	44
MAXIMUM RATE	1.07	51	54	105
STANDARD DEVIATION	0.70			
EQUATION: T = 0.23 * (X) + 50.01	$R^2 = 0.78$	36	37	73
LARGEST OF AVERAGE OR EQUATION		36	37	73
				EPS GROUP

		DWELLIN		
SIZE		/ (
				TOTAL
		ENTERING	EXITING	TOTAL
	055	50%	50%	
	355			
	198	100	400	000
	4.31	168	168	336
AVERAGE RATE	9.52	372	3/1	/43
	21.85	852	852	1,704
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	418	418	836
LARGEST OF AVERAGE OR EQUATION		418	418	836
AM PEAK HOUR ADJACENT STREET		25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194			
MINIMUM RATE	0.33	7	19	26
AVERAGE RATE	0.75	15	44	59
MAXIMUM RATE	2.27	44	133	177
STANDARD DEVIATION	0.90			
EQUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	16	48	64
LARGEST OF AVERAGE OR EQUATION		16	48	64
AM PEAK HOUR GENERATOR		26%	74%	
NUMBER OF STUDIES	343			
AVERAGE SIZE	180			
MINIMUM RATE	0.33	7	19	26
AVERAGE RATE	0.77	16	44	60
MAXIMUM RATE	2.27	46	131	177
STANDARD DEVIATION	0.91			
EQUATION: T = 0.70 * (X) + 12.12	$R^2 = 0.89$	17	50	67
LARGEST OF AVERAGE OR EQUATION	11 - 0.00	17	50	67
PM PEAK HOUR ADJACENT STREET		63%	37%	
NUMBER OF STUDIES	321			
AVERAGE SIZE	207			
	0.42	21	12	33
AVERAGE RATE	1.00	49	29	78
MAXIMUM RATE	2.98	146	86	232
STANDARD DEVIATION	1.05			202
FOUATION N (T) = 0.90 * N(X) + 0.51	$R^2 = 0.01$	53	.31	84
LARGEST OF AVERAGE OR FOLIATION	IX = 0.31	53	31	84
PM PEAK HOUR GENERATOR		64%	36%	04
NUMBER OF STUDIES	362	0,40	0070	
	17/			
	0.42	21	12	33
	1 02	51	20	80 80
	2.02	1/10	23	222
	2.30	140	04	232
		EE	24	06
EQUATION. LN (1) = 0.88 [°] LN(X) + 0.62	K⁻ = 0.91	55 FF	31	00
		55	51	80
				C EPS
				• anvur

PROJECT	ANDERSON RUSSELL			
PARCEI	PARCEL V			
ITE LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
		DWELLIN		
SIZE		7	8	
		-	TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	208	207	415
AVERAGE RATE	9.91	387	386	773
MAXIMUM RATE	15.25	595	595	1,190
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	403	403	806
LARGEST OF AVERAGE OR EQUATION		403	403	806
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	21	18	39
AVERAGE RATE	0.93	39	34	73
MAXIMUM RATE	1.75	74	63	137
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	42	36	78
LARGEST OF AVERAGE OR EQUATION		42	36	78
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	185	185	370
AVERAGE RATE	8.62	336	336	672
MAXIMUM RATE	12.31	480	480	960
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	337	336	673
LARGEST OF AVERAGE OR EQUATION		337	336	673
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	23	20	43
AVERAGE RATE	0.86	36	31	67
MAXIMUM RATE	1.48	61	54	115
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	38	34	72
LARGEST OF AVERAGE OR EQUATION		38	34	72

	 			
		SINGLE FA	MILY - 210	
	I	DWELLIN	GUNITS	
SIZE		<u></u>		
l IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	1		TRIPS	
	<u>، </u>	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	<u>ا</u>	50%	50%	·
NUMBER OF STUDIES	355	i	. I	1
AVERAGE SIZE	198	┫		
MINIMUM RATE	4.31	237	237	474
AVERAGE RATE	9.52	524	523	1,047
MAXIMUM RATE	21.85	1,202	1,202	2,404
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	573	573	1,146
LARGEST OF AVERAGE OR EQUATION		573	573	1,146
AM PEAK HOUR ADJACENT STREET	! !	25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194	l	·	
MINIMUM RATE	0.33	9	27	36
AVERAGE RATE	0.75	21	62	83
MAXIMUM RATE	2.27	63	187	250
STANDARD DEVIATION	0.90			
FOUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	22	65	87
LARGEST OF AVERAGE OR EQUATION	<u> </u>	22	65	87
AM PEAK HOUR GENERATOR	īī	26%	74%	
NUMBER OF STUDIES	343	L		
AV/FRAGE SIZE	180	1		. I
	0.33	9	27	36
AV/FRAGE RATE	0.77	22	63	85
	2 27	65	185	250
STANDARD DEVIATION	0.91	l		
$E \cap I \wedge T \cap N + T = 0.70 * (X) + 12.12$	$D^2 = 0.89$	23	66	80
LARGEST OF AVERAGE OR FOLIATION	K = 0.09	23	200	<u>80</u>
	Y	63%	27%	03
	221	0370	3170	·
	J∠1 207	1 1	·	· •
	0.42	20	17	16
	0.42 1.00	23	11	40 110
	1.00	207	41	110
	2.90	201	121	320
	CU.1	70	40	444
EQUATION: $Lin(1) = 0.90$ $Lin(A) \pm 0.91$	R ⁻ = 0.91	12	42	114
	·			114
		64%	36%	·
	362	4		
	1/4		47	40
	0.42	29	1/	46
AVERAGE RATE	1.02	/2	40	112
MAXIMUM RATE	2.98	210	118	328
STANDARD DEVIATION	1.05			
EQUATION: LN (T) = 0.88 * LN(X) + 0.62	$R^2 = 0.91$	74	42	116
LARGEST OF AVERAGE OR EQUATION	!	74	42	116
	_	_	_	C EPS
				GROUP

PROJECT				
PARCEL	PARCEL W			
ITE LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		11	0	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	293	292	585
AVERAGE RATE	9.91	545	545	1,090
MAXIMUM RATE	15.25	839	839	1,678
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	555	554	1,109
LARGEST OF AVERAGE OR EQUATION		555	554	1,109
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	30	25	55
AVERAGE RATE	0.93	55	47	102
MAXIMUM RATE	1.75	104	89	193
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	58	49	107
LARGEST OF AVERAGE OR EQUATION		58	49	107
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	261	260	521
AVERAGE RATE	8.62	474	474	948
MAXIMUM RATE	12.31	677	677	1,354
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	475	474	949
LARGEST OF AVERAGE OR EQUATION		475	474	949
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	32	29	61
AVERAGE RATE	0.86	50	45	95
MAXIMUM RATE	1.48	86	77	163
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	52	46	98
LARGEST OF AVERAGE OR EQUATION		52	46	98
				C EPS GROUP

	 			
		SINGLE FA	MILY - 210	/
	I	DWELLIN	GUNITS	
SIZE	 	11	4	/
l IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	1		TRIPS	=
	·'	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	<u>ا ا</u>	50%	50%	ı ———.
NUMBER OF STUDIES	355	4	·	1
AVERAGE SIZE	198	I	·	·
MINIMUM RATE	4.31	246	245	491
AVERAGE RATE	9.52	543	542	1,085
MAXIMUM RATE	21.85	1,246	1,245	2,491
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	593	592	1,185
LARGEST OF AVERAGE OR EQUATION		593	592	1,185
AM PEAK HOUR ADJACENT STREET	! !	25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194	l	·	·
MINIMUM RATE	0.33	10	28	38
AVERAGE RATE	0.75	22	64	86
MAXIMUM RATE	2.27	65	194	259
STANDARD DEVIATION	0.90			
FOUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	23	67	90
LARGEST OF AVERAGE OR EQUATION	<u> </u>	23	67	90
AM PEAK HOUR GENERATOR	īī	26%	74%	
NUMBER OF STUDIES	343	L		· · · · · · · · · · · · · · · · · · ·
AV/FRAGE SIZE	180	1		1
	0.33	10	28	38
AV/FRAGE RATE	0.77	23	65	88
	2 27	67	192	259
STANDARD DEVIATION	0.91	↓ †		
$F \cap I \Delta T \cap N! T = 0.70 * (X) + 12.12$	$D^2 = 0.89$	24	68	92
ADGEST OF AVERAGE OR FOLIATION	$\mathbf{K} = 0.03$	24	83	Q2
	·	63%	27%	<u>JL</u>
	221	0070	J1 /0	· · · · · · · · · · · · · · · · · · ·
	32 i 207	1 1	·	1
	0.42	30	10	10
	0.42 1.00	30 70	10	40 11 <i>1</i>
	1.00	214	42	240
	2.90	214	120	340
	CU.1	74	A A	440
EQUATION: $Lin(1) = 0.90 Lin(A) + 0.91$	R ⁻ = 0.91	74	44	110
	·		44	118
		64%	36%	ı — – – – – – – – – – – – – – – – – – –
	362	4		1
	1/4			40
	0.42	31	1/	48
AVERAGE RATE	1.02	74	42	116
MAXIMUM RATE	2.98	218	122	340
STANDARD DEVIATION	1.05			
EQUATION: LN (T) = 0.88 * LN(X) + 0.62	$R^2 = 0.91$	77	43	120
LARGEST OF AVERAGE OR EQUATION	!	77	43	120
	_	_	_	₹ EPS
				GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL	PARCEL X			
ITE LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
INDEPENDENT VARIABLE		DWELLIN		
SIZE		11	4	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	303	303	606
AVERAGE RATE	9.91	565	565	1,130
MAXIMUM RATE	15.25	870	869	1,739
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	574	573	1,147
LARGEST OF AVERAGE OR EQUATION		574	573	1,147
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	31	26	57
AVERAGE RATE	0.93	57	49	106
MAXIMUM RATE	1.75	108	92	200
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	59	51	110
LARGEST OF AVERAGE OR EQUATION		59	51	110
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	270	270	540
AVERAGE RATE	8.62	492	491	983
MAXIMUM RATE	12.31	702	701	1,403
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	492	491	983
LARGEST OF AVERAGE OR EQUATION		492	491	983
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	33	30	63
AVERAGE RATE	0.86	52	46	98
MAXIMUM RATE	1.48	90	79	169
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	54	47	101
LARGEST OF AVERAGE OR EQUATION		54	47	101
				EPS BROUP

		ANDERSON		
		SINGLE FA		
		DWELLIN		
SIZE		10	9	
			TRIPS	70741
		ENTERING	EXITING	TOTAL
		50%	50%	
NUMBER OF STUDIES	355			
AVERAGE SIZE	198			
MINIMUM RATE	4.31	235	235	470
AVERAGE RATE	9.52	519	519	1,038
MAXIMUM RATE	21.85	1,191	1,191	2,382
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	569	568	1,137
LARGEST OF AVERAGE OR EQUATION		569	568	1,137
AM PEAK HOUR ADJACENT STREET		25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194			
MINIMUM RATE	0.33	9	27	36
AVERAGE RATE	0.75	21	61	82
MAXIMUM RATE	2.27	62	185	247
STANDARD DEVIATION	0.90			
EQUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	22	64	86
LARGEST OF AVERAGE OR EQUATION		22	64	86
AM PEAK HOUR GENERATOR		26%	74%	
NUMBER OF STUDIES	343			
AVERAGE SIZE	180			
MINIMUM RATE	0.33	9	27	36
AVERAGE RATE	0.77	22	62	84
MAXIMUM RATE	2.27	64	183	247
STANDARD DEVIATION	0.91			
EQUATION: T = 0.70 * (X) + 12.12	$R^2 = 0.89$	23	65	88
LARGEST OF AVERAGE OR EQUATION		23	65	88
PM PEAK HOUR ADJACENT STREET		63%	37%	
NUMBER OF STUDIES	321			
AVERAGE SIZE	207			
MINIMUM RATE	0.42	29	17	46
AVERAGE RATE	1.00	69	40	109
MAXIMUM RATE	2.98	205	120	325
STANDARD DEVIATION	1.05		-	
EQUATION: $LN(T) = 0.90 * LN(X) + 0.51$	$R^2 = 0.91$	72	42	114
LARGEST OF AVERAGE OR EQUATION	1 0.01	72	42	114
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	362	0170	0070	
AVERAGE SIZE	174			
MINIMUM RATE	0.42	29	17	46
AVERAGE RATE	1 02	71	40	111
	2 98	208	117	325
	1 05	200	117	020
$F \cap \Delta T \cap N \cdot N \langle T \rangle = 0.88 * N \langle Y \rangle \pm 0.62$	$P^2 = 0.04$	7/	/1	115
	K = 0.91	74	41	115
LANGEST OF AVERAGE OR EQUATION		/4	41	
				GEPS GROUP

		ANDERSON	IRUSSELL	
PARCEL	PARCEL Y			
ITE LAND USE CATEGORY AND CODE	SINGLE FAMILY - 210			
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		10	9	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	290	290	580
AVERAGE RATE	9.91	540	540	1,080
MAXIMUM RATE	15.25	831	831	1,662
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	550	550	1,100
LARGEST OF AVERAGE OR EQUATION		550	550	1,100
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	30	25	55
AVERAGE RATE	0.93	55	46	101
MAXIMUM RATE	1.75	103	88	191
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	57	49	106
LARGEST OF AVERAGE OR EQUATION		57	49	106
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	259	258	517
AVERAGE RATE	8.62	470	470	940
MAXIMUM RATE	12.31	671	671	1,342
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	470	470	940
LARGEST OF AVERAGE OR EQUATION		470	470	940
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	32	28	60
AVERAGE RATE	0.86	50	44	94
MAXIMUM RATE	1.48	85	76	161
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	51	46	97
LARGEST OF AVERAGE OR EQUATION		51	46	97
				C EPS

	ł			
	ł	SINGLE FA	MILY - 210	
	l			
SIZE	 	<u></u>	2	
l IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	1 '		TRIPS	
	<u>بــــــــــــــــــــــــــــــــــــ</u>	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	<u>ا '</u>	50%	50%	ı
NUMBER OF STUDIES	355	4	·	1
AVERAGE SIZE	198	I	_	_
MINIMUM RATE	4.31	242	241	483
AVERAGE RATE	9.52	533	533	1,066
MAXIMUM RATE	21.85	1,224	1,223	2,447
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	583	583	1,166
LARGEST OF AVERAGE OR EQUATION	l'	583	583	1,166
AM PEAK HOUR ADJACENT STREET	'	25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194	l	·	·
MINIMUM RATE	0.33	9	28	37
AVERAGE RATE	0.75	21	63	84
MAXIMUM RATE	2.27	64	190	254
STANDARD DEVIATION	0.90			
FOUNTION: $T = 0.70 * (X) + 9.74$	$R^2 = 0.89$	22	66	88
LARGEST OF AVERAGE OR EQUATION	r <u> </u>	22	66	88
AM PEAK HOUR GENERATOR	· · · · · · · · · · · · · · · · · · ·	26%	74%	
NUMBER OF STUDIES	343	L		· · · · · · · · · · · · · · · · · · ·
AV/FRAGE SIZE	180	1		1
	0.33	10	27	37
AV/FRAGE RATE	0.25	22	64	86
	2 27	66	188	254
STANDARD DEVIATION	0.91	l		
$F \cap I \Delta T \cap N! T = 0.70 * (X) + 12.12$	$D^2 = 0.89$	24	67	91
LADGEST OF AVERAGE OR FOLIATION	$\mathbf{K} = 0.05$	24	67	Q1
	·	63%	27%	
	221	0070	J1 /0	
	207	1 1	·	1
	0.42	30	17	17
	1.00	71	11	47 110
	1.00	210	41	11Z 224
	2.90	210	124	334
		70	40	110
EQUATION: Lin (1) = 0.90 Lin(Λ) + 0.91	$R^2 = 0.91$	13	43	110
	<u> </u> /	/3	43	110
		64%	36%	· · · · · · · · · · · · · · · · · · ·
	362	4		
	1/4		47	4-7
	0.42	30	1/	47
AVERAGE RATE	1.02	/3	41	114
MAXIMUM RATE	2.98	214	120	334
STANDARD DEVIATION	1.05			· · · -
EQUATION: LN (T) = $0.88 * LN(X) + 0.62$	$R^2 = 0.91$	76	42	118
LARGEST OF AVERAGE OR EQUATION	'	76	42	118
				EPS
				GROUP

PROJECT	ANDERSON RUSSELL			
PARCEL		PARC	ELZ	
ITE LAND USE CATEGORY AND CODE	SINGLE FAMILY - 210			
INDEPENDENT VARIABLE		DWELLIN	G UNITS	
SIZE		11	2	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	298	298	596
AVERAGE RATE	9.91	555	555	1,110
MAXIMUM RATE	15.25	854	854	1,708
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	564	564	1,128
LARGEST OF AVERAGE OR EQUATION		564	564	1,128
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	30	26	56
AVERAGE RATE	0.93	56	48	104
MAXIMUM RATE	1.75	106	90	196
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	58	50	108
LARGEST OF AVERAGE OR EQUATION		58	50	108
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	266	265	531
AVERAGE RATE	8.62	483	482	965
MAXIMUM RATE	12.31	690	689	1,379
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	483	483	966
LARGEST OF AVERAGE OR EQUATION		483	483	966
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	33	29	62
AVERAGE RATE	0.86	51	45	96
MAXIMUM RATE	1.48	88	78	166
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = $0.91 * LN(X) + 0.31$	$R^2 = 0.88$	53	47	100
LARGEST OF AVERAGE OR EQUATION		53	47	100
EPS BROUP				

	SINGLE FAMILY - 210			
			G UNITS	
SIZE		00		
				τοται
		ENTERING	EATTING 50%	TOTAL
	255	50 %	50 %	
	109			
	190	196	195	271
	4.51	100	100	910
	9.52	410 940	409	1 870
	21.00	940	939	1,079
STANDARD DEVIATION EQUATION: $I N (T) = 0.02 * I N(X) + 2.72$	5.70 D ² 0.05	457	457	014
EQUATION. LN $(1) = 0.92$ LN $(X) + 2.72$	$R^{-} = 0.95$	437 457	407 457	914
		457	437	914
	202	23%	75%	
	292			
	194	7	21	20
	0.33	1	21	20
	0.75	16	49	60
	2.27	49	146	195
	0.90	40	50	70
EQUATION: $I = 0.70^{\circ} (X) + 9.74$	$R^2 = 0.89$	18	52	70
		18	52	70
	242	26%	74%	
	343			
	180	7	04	00
	0.33	(21	28
	0.77	17	49	66
	2.27	51	144	195
	0.91	10	50	70
EQUATION: $I = 0.70^{\circ} (X) + 12.12$	$R^2 = 0.89$	19	53	72
LARGEST OF AVERAGE OR EQUATION		19	53	72
	004	63%	37%	
	321			
	207	00	40	20
	0.42	23	13	30
	1.00	54	32	86
	2.98	161	95	256
	1.05	50	0.4	
EQUATION: $LN(1) = 0.90^{\circ} LN(X) + 0.51^{\circ}$	$R^2 = 0.91$	58	34	92
LARGEST OF AVERAGE OR EQUATION		58	34	92
	000	64%	36%	
	362			
	1/4	00	40	22
	0.42	23	13	30
	1.02	56	32	88
	2.98	164	92	256
	1.05		<u></u>	<u></u>
EQUATION: $LN(1) = 0.88 * LN(X) + 0.62$	R ² = 0.91	60	34	94
LARGEST OF AVERAGE OR EQUATION		60	34	94
				C EPS
				₩ 6KUUP

PROJECT				
PARCEI	PARCEL AA			
ITE LAND USE CATEGORY AND CODE	SINGLE FAMILY - 210			
SIZE		8	6	
OILL				
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	229	229	458
AVERAGE RATE	9.91	426	426	852
MAXIMUM RATE	15.25	656	656	1,312
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	441	441	882
LARGEST OF AVERAGE OR EQUATION		441	441	882
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	23	20	43
AVERAGE RATE	0.93	43	37	80
MAXIMUM RATE	1.75	82	69	151
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	46	39	85
LARGEST OF AVERAGE OR EQUATION		46	39	85
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	204	204	408
AVERAGE RATE	8.62	371	370	741
MAXIMUM RATE	12.31	530	529	1,059
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	371	371	742
LARGEST OF AVERAGE OR EQUATION		371	371	742
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	25	22	47
AVERAGE RATE	0.86	39	35	74
MAXIMUM RATE	1.48	67	60	127
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	42	37	79
LARGEST OF AVERAGE OR EQUATION		42	37	79

	ANDERSON RUSSELL			
	 			
	i	SINGLE FA		
	 	DWELLIN		
SIZE	 	10	<u>/8</u>	/
	1 '		I RIPS	=
	ب '	ENTERING	EXITING	TOTAL
WEEKDAY DAILY	I′	50%	50%	ı
NUMBER OF STUDIES	355			
AVERAGE SIZE	198	┫		,Į
MINIMUM RATE	4.31	233	232	465
AVERAGE RATE	9.52	514	514	1,028
MAXIMUM RATE	21.85	1,180	1,180	2,360
STANDARD DEVIATION	3.70			
EQUATION: LN (T) = 0.92 * LN(X) + 2.72	$R^2 = 0.95$	564	563	1,127
LARGEST OF AVERAGE OR EQUATION	L'	564	563	1,127
AM PEAK HOUR ADJACENT STREET	!	25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194		·]	·
MINIMUM RATE	0.33	9	27	36
AVERAGE RATE	0.75	20	61	81
MAXIMUM RATE	2.27	61	184	245
STANDARD DEVIATION	0.90		_	1
FOUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	21	64	85
I ARGEST OF AVERAGE OR EQUATION	<u> </u>	21	64	85
AM PEAK HOUR GENERATOR	,	26%	74%	
NUMBER OF STUDIES	343			
AV/FRAGE SIZE	180	1 1	·	1 I
	0.33	9	27	36
AV/FRAGE RATE	0.77	22	61	83
MAXIMIM RATE	2 27	64	181	245
STANDARD DEVIATION	0.91	<u>↓ ~ ` </u> †	· · · ·	
$F \cap I \Delta T \cap N! T = 0.70 * (X) + 12.12$	$D^2 = 0.89$	23	65	88
APGEST OF AVERAGE OR EQUATION	<u>N - 0.00</u>	23	65	88
		63%	37%	
	321	0070	5170	
	207	1		
	0.42	28	17	45
	1.00	20 69	40	40 100
	1.00	202	40	100
	2.90	203	113	322
	1.00 5 ² 0.04	74	40	110
EQUATION: Liv $(1) = 0.90$ Liv $(x) + 0.91$	R ² = 0.91		42	113
	·/		42	113
PM PEAK HOUR GENERATOR		64%	36%	
	362	4		1
	1/4			·
MINIMUM RATE	0.42	29	16	45
AVERAGE RATE	1.02	70	40	110
MAXIMUM RATE	2.98	206	116	322
STANDARD DEVIATION	1.05	l		J
EQUATION: LN (T) = 0.88 * LN(X) + 0.62	$R^2 = 0.91$	73	41	114
LARGEST OF AVERAGE OR EQUATION	<u> </u>	73	41	114
				✓ EPS
				GROUP
PROJECT		ANDERSON	RUSSELL	
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PARCEL		PARCE	EL BB	
ITE LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
		DWELLIN		
SIZE		10	8	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	288	287	575
AVERAGE RATE	9.91	535	535	1,070
MAXIMUM RATE	15.25	824	823	1,647
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	545	545	1,090
LARGEST OF AVERAGE OR EQUATION		545	545	1,090
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	29	25	54
AVERAGE RATE	0.93	54	46	100
MAXIMUM RATE	1.75	102	87	189
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	57	48	105
LARGEST OF AVERAGE OR EQUATION		57	48	105
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	256	256	512
AVERAGE RATE	8.62	466	465	931
MAXIMUM RATE	12.31	665	664	1,329
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	466	465	931
LARGEST OF AVERAGE OR EQUATION		466	465	931
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	31	28	59
AVERAGE RATE	0.86	49	44	93
MAXIMUM RATE	1.48	85	75	160
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	51	46	97
LARGEST OF AVERAGE OR EQUATION		51	46	97
				EPS GROUP

		DWELLIN		
SIZE		44		
				TOTAL
		ENTERING	EXITING	TOTAL
	055	50%	50%	
	355			
	198	04	00	404
	4.31	91	90	181
AVERAGE RATE	9.52	200	200	400
	21.85	459	459	918
STANDARD DEVIATION	3.70			
EQUATION: $LN(T) = 0.92 * LN(X) + 2.72$	$R^2 = 0.95$	237	236	473
LARGEST OF AVERAGE OR EQUATION		237	236	473
AM PEAK HOUR ADJACENT STREET		25%	75%	
NUMBER OF STUDIES	292			
AVERAGE SIZE	194			
MINIMUM RATE	0.33	4	10	14
AVERAGE RATE	0.75	8	24	32
MAXIMUM RATE	2.27	24	71	95
STANDARD DEVIATION	0.90			
EQUATION: T = 0.70 * (X) + 9.74	$R^2 = 0.89$	10	29	39
LARGEST OF AVERAGE OR EQUATION		10	29	39
AM PEAK HOUR GENERATOR		26%	74%	
NUMBER OF STUDIES	343			
AVERAGE SIZE	180			
MINIMUM RATE	0.33	4	10	14
AVERAGE RATE	0.77	8	24	32
MAXIMUM RATE	2.27	25	70	95
STANDARD DEVIATION	0.91			
EQUATION: T = 0.70 * (X) + 12.12	$R^2 = 0.89$	11	31	42
LARGEST OF AVERAGE OR EQUATION	11 - 0.00	11	31	42
PM PEAK HOUR ADJACENT STREET		63%	37%	
NUMBER OF STUDIES	321		0170	
AVERAGE SIZE	207			
	0.42	11	7	18
AVERAGE RATE	1.00	26	16	42
	2.98	79	46	125
STANDARD DEVIATION	1 05	, ,	10	120
FOUATION N (T) = 0.90 * N(X) + 0.51	$R^2 = 0.01$	30	18	48
LARGEST OF AVERAGE OR FOLIATION	1 - 0.31	30	18	48
PM PEAK HOUR GENERATOR		64%	36%	
NUMBER OF STUDIES	362	0,40	0070	
	17/			
	0.42	12	6	18
	1 02	28	15	/3
	2.02	20	15	125
	2.30	00	40	120
		22	4.0	50
EQUATION. LN (1) = 0.88 [°] LN(X) + 0.62	K⁻ = 0.91	32	18	50
LARGEST OF AVERAGE OR EQUATION		52	18	50
				EPS GROUP

PROJECT		ANDERSON		
PARCEI		PARCI	EL CC	
ITE LAND USE CATEGORY AND CODE		SINGLE FA	 MILY - 210	
		DWELLIN		
SIZE		4	2	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
NUMBER OF STUDIES	77			
AVERAGE SIZE	215			
MINIMUM RATE	5.32	112	111	223
AVERAGE RATE	9.91	208	208	416
MAXIMUM RATE	15.25	321	320	641
STANDARD DEVIATION	3.72			
EQUATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	227	226	453
LARGEST OF AVERAGE OR EQUATION		227	226	453
PEAK HOUR GENERATOR		54%	46%	
NUMBER OF STUDIES	54			
AVERAGE SIZE	215			
MINIMUM RATE	0.50	11	10	21
AVERAGE RATE	0.93	21	18	39
MAXIMUM RATE	1.75	40	34	74
STANDARD DEVIATION	0.99			
EQUATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	25	21	46
LARGEST OF AVERAGE OR EQUATION		25	21	46
SUNDAY DAILY		50%	50%	
NUMBER OF STUDIES	73			
AVERAGE SIZE	218			
MINIMUM RATE	4.74	100	99	199
AVERAGE RATE	8.62	181	181	362
MAXIMUM RATE	12.31	259	258	517
STANDARD DEVIATION	3.36			
EQUATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	181	181	362
LARGEST OF AVERAGE OR EQUATION		181	181	362
PEAK HOUR GENERATOR		53%	47%	
NUMBER OF STUDIES	53			
AVERAGE SIZE	212			
MINIMUM RATE	0.55	12	11	23
AVERAGE RATE	0.86	19	17	36
MAXIMUM RATE	1.48	33	29	62
STANDARD DEVIATION	0.95			
EQUATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	22	19	41
LARGEST OF AVERAGE OR EQUATION		22	19	41

			5 UNITS	
SILE		0,		
				TUTAL
	055	50%	50%	
	355	1		
	198	170	470	050
	4.31	1/9	1/9	358
AVERAGE RATE	9.52	395	395	790
MAXIMUM RATE	21.85	907	907	1,814
STANDARD DEVIATION	3.70			
EQUATION: $LN(T) = 0.92 * LN(X) + 2.72$	$R^2 = 0.95$	443	442	885
LARGEST OF AVERAGE OR EQUATION		443	442	885
AM PEAK HOUR ADJACENT STREET		25%	75%	
NUMBER OF STUDIES	292	ſ I	I	
AVERAGE SIZE	194			
MINIMUM RATE	0.33	7	20	27
AVERAGE RATE	0.75	16	46	62
MAXIMUM RATE	2.27	47	141	188
STANDARD DEVIATION	0.90			
EQUATION: T = 0.70 * (X) + <u>9.74</u>	$R^2 = 0.89$	17	51	68
LARGEST OF AVERAGE OR EQUATION		17	51	68
AM PEAK HOUR GENERATOR		26%	74%	
NUMBER OF STUDIES	343			
AVERAGE SIZE	180	1		
MINIMUM RATE	0.33	7	20	27
AVERAGE RATE	0.77	17	47	64
MAXIMUM RATE	2.27	49	139	188
STANDARD DEVIATION	0.91			-
FOUATION: $T = 0.70 * (X) + 12.12$	$R^2 = 0.89$	18	52	70
I ARGEST OF AVERAGE OR EQUATION	- 0.00	18	52	70
PM PFAK HOUR ADJACENT STREET		63%	37%	
NI IMBER OF STUDIES	321			
AV/FRAGE SIZE	207	1		
MINIMI IM RATE	0.42	22	13	35
	1 00	52	31	83
	2.98	156	Q1	247
	1 05	100		۲۱ ک
EQUATION: IN (T) = $0.90 \times 1.0(X) + 0.51$	$D^2 = 0.01$	56	22	80
	K = U.91	00 88	33 22	00 20
		6/0/	33 26%	03
	360	U++ /u	3070	
	302 17/	1		
	0 42	22	12	25
	0.42	<u> </u>	10	30 05
	1.02	54	31	047
	2.98	158	89	247
	1.05			
EQUATION: LN (T) = $0.88 \wedge LN(X) + 0.62$	R [∠] = 0.91	58	33	91
LARGEST OF AVERAGE OR EQUATION		58	33	91
				EPS
				W GROUP

PROJECT		ANDERSON	RUSSELL	
PARCEL		PARCI	EL DD	
LAND USE CATEGORY AND CODE		SINGLE FA	MILY - 210	
		DWELLIN		
SIZE		8	3	
			TRIPS	
	RATE	ENTERING	EXITING	SUM
SATURDAY DAILY		50%	50%	
MBER OF STUDIES	77			
ERAGE SIZE	215			
IIMUM RATE	5.32	221	221	442
ERAGE RATE	9.91	412	411	823
XIMUM RATE	15.25	633	633	1,266
ANDARD DEVIATION	3.72			
UATION: LN (T) = 0.93 * LN(X) + 2.64	$R^2 = 0.92$	427	427	854
GEST OF AVERAGE OR EQUATION		427	427	854
PEAK HOUR GENERATOR		54%	46%	
MBER OF STUDIES	54			
ERAGE SIZE	215			
IIMUM RATE	0.50	23	19	42
ERAGE RATE	0.93	42	35	77
XIMUM RATE	1.75	78	67	145
ANDARD DEVIATION	0.99			
UATION: T = 0.89 * (X) + 8.77	$R^2 = 0.91$	45	38	83
GEST OF AVERAGE OR EQUATION		45	38	83
SUNDAY DAILY		50%	50%	
MBER OF STUDIES	73			
ERAGE SIZE	218			
IIMUM RATE	4.74	197	196	393
ERAGE RATE	8.62	358	357	715
XIMUM RATE	12.31	511	511	1,022
ANDARD DEVIATION	3.36			
UATION: T = 8.63 * (X) - 0.63	$R^2 = 0.93$	358	358	716
GEST OF AVERAGE OR EQUATION		358	358	716
PEAK HOUR GENERATOR		53%	47%	
MBER OF STUDIES	53			
ERAGE SIZE	212			
IIMUM RATE	0.55	24	22	46
ERAGE RATE	0.86	38	33	71
XIMUM RATE	1.48	65	58	123
ANDARD DEVIATION	0.95			
UATION: LN (T) = 0.91 * LN(X) + 0.31	$R^2 = 0.88$	40	36	76
GEST OF AVERAGE OR EQUATION		40	36	76
				EPS BROUP
UATION: LN (T) = 0.91 * LN(X) + 0.31 GEST OF AVERAGE OR EQUATION	$R^2 = 0.88$	40 40	36 36	

ATTACHMENT D LEVEL-OF-SERVICE ANALYSIS OUTPUT SHEETS



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ľ	<u>††</u>	1	٦	et		ሻሻ	et F			\$	
Traffic Volume (vph)	20	424	1203	399	241	1	333	2	120	1	3	27
Future Volume (vph)	20	424	1203	399	241	1	333	2	120	1	3	27
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999			0.852			0.881	
Flt Protected	0.950			0.950			0.950				0.999	
Satd. Flow (prot)	1770	3539	1583	1770	1861	0	3433	1587	0	0	1639	0
Flt Permitted	0.594			0.272		-	0.950			-	0.999	
Satd. Flow (perm)	1106	3539	1583	507	1861	0	3433	1587	0	0	1639	0
Satd. Flow (RTOR)		174	/65				070	133	100		30	
Adj. Flow (vph)	22	4/1	1337	443	268	1	370	2	133	1	3	30
Lane Group Flow (vph)	22	4/1	1337	443	269	0	3/0	135	0	0	34	0
Turn Type	pm+pt	NA	Free	pm+pt	NA		Split	NA		Split	NA	
Protected Phases	/	4	E.e.e.	3	8		2	2		6	6	
Permilled Phases	4	<u> 11 г</u>	Free	8	25.0		22.0	22.0		<u>ээ г</u>	<u>ээ г</u>	
Total Spill (S)	9.5 4 E	22.5 4 E		22.0	35.0		23.U	23.U		22.5	22.5 4 E	
Total Lost Time (S)	4.5	4.0 14.0	75 1	4.5	4.0 22.0		4.0 10 E	4.0 10 E			4.5	
Actuated a/C Datio	19.8	14.8	/ 5. I 1.00	30.8	33.Z		10.0	10.0			0.2	
Actualeu y/C Ratio	0.20	0.20	0.04	0.49	0.44		0.25	0.20			0.00	
Control Dolay	0.07	22.2	6.2	28.0	0.55		0.44	6.27			17.0	
	12.5	0.0	0.0	20.0	0.4		20.4	0.0			0.0	
Total Delay	12.3	22.2	6.3	28.0	16 /		26.4	6.8			17.0	
	12.5 R	55.5 C	Δ	20.0	10.4 R		20.4	0.0			17.7 B	
Approach Delay	U	13.3	7.	U	23.6		U	21.2			17 9	
Approach LOS		10.0 B			20.0 C			C.			B	
Queue Length 50th (ft)	5	106	0	130	69		74	1			2	
Oueue Length 95th (ft)	16	159	0	#284	156		122	43			28	
Internal Link Dist (ft)		608	-		332			694			269	
Turn Bay Length (ft)												
Base Capacity (vph)	335	850	1583	543	828		847	492			416	
Starvation Cap Reductn	0	0	0	0	0		0	0			0	
Spillback Cap Reductn	0	0	0	0	0		0	0			0	
Storage Cap Reductn	0	0	0	0	0		0	0			0	
Reduced v/c Ratio	0.07	0.55	0.84	0.82	0.32		0.44	0.27			0.08	
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 75.1												
Control Type: Actuated-Unco	oordinated											
Maximum v/c Ratio: 0.84												
Intersection Signal Delay: 17	7.1			In	tersectior	n LOS: B						
Intersection Capacity Utilizat	tion 61.2%			IC	CU Level of	of Service	B					
Analysis Period (min) 15												
# 95th percentile volume e	exceeds ca	oacity, qu	ieue may	be longe	r.							
Queue shown is maximu	m after two	cycles.										

Splits and Phases: 1: Anderson Road & Maricopa-Casa Grande Highway

2020 with Site Phase I AM

▲ Ø2	₩ø6	√ Ø3	<u>⊿</u> _{Ø4}
23 s	22.5 s	22 s	22.5 s
		▶ _{Ø7} ★ _{Ø8}	
		9.5 s 35 s	

	۶	-	*	4	+	*	≺	1	1	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	† †	1	۲	el el		ሻሻ	f,			\$	
Traffic Volume (vph)	18	318	927	308	452	1	1465	2	492	1	1	15
Future Volume (vph)	18	318	927	308	452	1	1465	2	492	1	1	15
Lane Util. Factor	1.00	0.95	1.00	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	1.00
Frt			0.850					0.851			0.879	
Flt Protected	0.950			0.950			0.950				0.997	
Satd. Flow (prot)	1770	3539	1583	1770	1863	0	3433	1585	0	0	1632	0
Flt Permitted	0.192			0.254			0.950				0.997	
Satd. Flow (perm)	358	3539	1583	473	1863	0	3433	1585	0	0	1632	0
Satd. Flow (RTOR)			866					547			17	
Adj. Flow (vph)	20	353	1030	342	502	1	1628	2	547	1	1	17
Lane Group Flow (vph)	20	353	1030	342	503	0	1628	549	0	0	19	0
Turn Type	pm+pt	NA	Free	pm+pt	NA		Split	NA		Split	NA	
Protected Phases	7	4		3	8		2	2		6	6	
Permitted Phases	4		Free	8								
Total Split (s)	9.5	26.1		27.4	44.0		74.0	74.0		22.5	22.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5			4.5	
Act Effct Green (s)	24.0	19.0	135.7	46.4	40.8		69.6	69.6			6.2	
Actuated g/C Ratio	0.18	0.14	1.00	0.34	0.30		0.51	0.51			0.05	
v/c Ratio	0.17	0.71	0.65	0.90	0.90		0.93	0.51			0.21	
Control Delay	35.1	64.4	2.1	64.2	66.1		41.2	3.3			33.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0		13.5	0.1			0.0	
Total Delay	35.1	64.4	2.1	64.2	66.1		54.8	3.4			33.3	
LOS	D	E	A	Ł	E		D	A			С	
Approach Delay		18.2			65.3			41.8			33.3	
Approach LOS	10	В	0	0.40	E		(00	D			C	
Queue Length 50th (ft)	12	157	0	243	441		688	1			2	_
Queue Length 95th (ft)	31	216	0	#399	#6/6		#891	60			30	
Internal Link Dist (ft)		608			332			694			269	
Turn Bay Length (ft)	445	F (0	4500	000	F (0		47/0	4070			001	
Base Capacity (vph)	115	563	1583	380	560		1/60	1079			231	
Starvation Cap Reductin	0	0	0	0	0		160	58			0	
Spiliback Cap Reductin	0	0	0	0	0		0	0			0	_
Storage Cap Reducin	0 17	0	0	0	0		1 00	0			0	
Reduced V/C Rallo	0.17	0.63	0.65	0.90	0.90		1.02	0.54			0.08	
Intersection Summary												
Cycle Length: 150	_											
Actuated Cycle Length: 135.	/											
Control Type: Actuated-Unco	pordinated											
Maximum v/c Ratio: 0.93												
Intersection Signal Delay: 38	3.8			In	tersection	1 LOS: D	-					_
Intersection Capacity Utilizat	ion $8/.1\%$			IC	CU Level of	of Service	εE					
Analysis Period (min) 15												
# 95th percentile volume e	xceeds cap	oacity, qu	ieue may	be longe	ſ .							
Queue snown is maximur	n aner two	cycles.										

Splits and Phases: 1: Anderson Road & Maricopa-Casa Grande Highway

2020 with Site Phase I PM

↑ _{Ø2}	Ø6	√ Ø3	<u>⊿</u> _{Ø4}
74 s	22.5 s	27.4 s	26.1 s
		▲ Ø7 ★ Ø8	
		9.5 s 44 s	

2020 with Site Phase I AM

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	N		5	*	*	1
Traffic Volume (vph)	208	8	33	247	788	819
Future Volume (vph)	208	8	33	247	788	819
Lane Util. Factor	0.97	0.95	1.00	1.00	1.00	1.00
Frt	0.994					0.850
Flt Protected	0.954		0.950			
Satd, Flow (prot)	3427	0	1770	1863	1863	1583
Flt Permitted	0.954		0.233			
Satd. Flow (perm)	3427	0	434	1863	1863	1583
Satd. Flow (RTOR)	7					910
Adj. Flow (vph)	231	9	37	274	876	910
Lane Group Flow (vph)	240	0	37	274	876	910
Turn Type	Perm		Perm	NA	NA	Perm
Protected Phases				2	6	
Permitted Phases	4		2	-	J	6
Total Split (s)	22.5		37.5	37.5	37.5	37.5
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Act Effet Green (s)	9.4		41.6	41.6	41.6	41.6
Actuated g/C Ratio	0.16		0.69	0.69	0.69	0.69
v/c Ratio	0.44		0.12	0.21	0.68	0.66
Control Delay	24.4		63	5.7	93	3 1
Oueue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	24.4		63	5.0	0.0	3.0 3.1
	۲.ד2 ۲		Δ	Δ	Δ	Δ
Approach Delay	24.4			57	61	Π
Approach LOS	24.4			Δ	Δ	
Oueue Length 50th (ft)	20		1	7/	1/12	0
Oueue Length 95th (ft)	65		21	128	200	22
Internal Link Dist (ff)	256		51	5/12	69/	JZ
Turn Bay Length (ft)	200			J42	074	
Base Canacity (whh)	1033		300	1201	1201	1376
Starvation Can Reductn	000		0	1271	1271	1370
Snillhack Can Reducth	0		0	0	0	0
Storage Can Reductin	0		0	0	0	0
Reduced v/c Ratio	0.23		0 12	0.21	0 68	0.66
	0.20		0.12	0.21	0.00	0.00
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset: 0 (0%), Referenced	to phase 2:	NBTL an	d 6:SBT, 1	Start of G	reen	
Control Type: Actuated-Coc	ordinated					
Maximum v/c Ratio: 0.68						
Intersection Signal Delay: 7	.9			In	tersectior	n LOS: A
Intersection Capacity Utiliza	ation 62.4%			IC	U Level	of Service
Analysis Period (min) 15						

Splits and Phases: 2: Anderson Road & Access 1



2020 with Site Phase I PM

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	NM		5	**	**	1
Traffic Volume (vph)	1006	40	25	953	618	619
Future Volume (vph)	1006	40	25	953	618	619
Lane Util. Factor	0.97	0.95	1.00	0.95	0.95	1.00
Frt	0.994					0.850
Flt Protected	0.954		0.950			
Satd. Flow (prot)	3427	0	1770	3539	3539	1583
Flt Permitted	0.954		0.334			
Satd. Flow (perm)	3427	0	622	3539	3539	1583
Satd. Flow (RTOR)	8					688
Adi, Flow (vph)	1118	44	28	1059	687	688
Lane Group Flow (vph)	1162	0	28	1059	687	688
Turn Type	Perm	-	Perm	NA	NA	Perm
Protected Phases				2	6	
Permitted Phases	4		2	-	v	6
Total Split (s)	30.0		30.0	30.0	30.0	30.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Act Effct Green (s)	24.3		26.7	26.7	26.7	26.7
Actuated g/C Ratio	0 40		0 44	0.44	0.44	0.44
v/c Ratio	0.10		0.10	0.67	0.44	0.63
Control Delay	22.4		11 3	18.7	12.9	4.2
Oueue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	22.4		11 3	18.7	12.9	4.2
	22.4		R	10.7 R	12.7 R	τ. <u>∠</u>
Annroach Delay	22 /		D	18.5	8.6	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
Approach LOS	22.4 C			10.5 R	Δ	
Oueue Length 50th (ft)	178		Q	227	88	0
Queue Length 95th (ft)	250		, m10	227	129	5/
Internal Link Dist (ft)	256		mito	5/2	69/	54
Turn Ray Length (ft)	200			J42	074	
Rase Canacity (unh)	1/61		277	1577	1577	1096
Starvation Can Doducto	1401 0		211	1377	1377	0001
Snillback Can Poductn	0		0	0	0	0
Storage Can Reductin	0		0	0	0	0
Doducod v/c Datio	0 0		0 10	0.67	0.44	0.62
KEUULEU VIL KAIIO	U.SU		0.10	0.07	0.44	0.03
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset: 0 (0%), Referenced	to phase 2:1	NBTL an	d 6:SBT,	Start of G	reen	
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.84						
Intersection Signal Delay: 1	16.0			In	tersectior	ו LOS: B
Intersection Capacity Utilization	ation 63.8%			IC	U Level	of Service
Analysis Period (min) 15						
m Volume for 95th percer	ntile queue is	s metere	d by upstr	eam sion	al.	
F	1		2.1.20	- 91		

Splits and Phases: 2: Anderson Road & Access 1



2020 with Site Phase I AM

	٦	$\mathbf{\hat{z}}$	1	1	Ŧ	-
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	NM		*	٨	*	1
Traffic Volume (vph)	187	12	49	93	59	737
Future Volume (vph)	187	12	47	93	59	737
Lane Litil Factor	0.97	0.95	1 00	1 00	1 00	1 00
Frt	0.77	0.75	1.00	1.00	1.00	0.850
Flt Protected	0.771		0.950			0.000
Satd Flow (prot)	3/20	0	1770	1863	1863	1583
Salu. Flow (prot)	0.055	0	0.714	1005	1003	1000
Fit Permitteu	0.900	0	0.714	1040	1040	1500
Salu. Flow (perm)	3420	0	1330	1803	1803	1583
Salo. Flow (RTOR)	11	10	E 4	100		819
Adj. Flow (vpn)	208	13	54	103	66	819
Lane Group Flow (vph)	221	0	54	103	66	819
Turn Type	Prot		Perm	NA	NA	Perm
Protected Phases	4			2	6	
Permitted Phases			2			6
Total Split (s)	22.6		37.4	37.4	37.4	37.4
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Act Effct Green (s)	9.0		42.0	42.0	42.0	42.0
Actuated g/C Ratio	0.15		0.70	0.70	0.70	0.70
v/c Ratio	0.42		0.06	0.08	0.05	0.61
Control Delay	24.1		3.5	3.5	2.9	2.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	24.1		3.5	3.5	2.9	2.8
LOS	<u>с</u>		A	A	A	A
Approach Delay	24.1			3.5	2.8	
Approach LOS	<u>с</u>			A	Α	
Queue Length 50th (ft)	36		5	9	4	11
Oueue Length 95th (ft)	60		15	24	т m0	61
Internal Link Dist (ft)	215		15	640	5/2	01
Turn Bay Longth (ft)	210			040	04Z	
Paso Capacity (upb)	1020		021	1204	1204	1252
Dase Capacity (VPII)	1039		421	1304	1304	1303
Starvation Cap Reductin	U		0	0	U	0
Spillback Cap Reductin	U		U	0	U	U
Storage Cap Reductin	U		0	0	0	0
Reduced v/c Ratio	0.21		0.06	0.08	0.05	0.61
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset: 0 (0%), Referenced	to phase 2:	NBTL an	d 6:SBT, 3	Start of G	reen	
Control Type: Actuated-Coo	ordinated					
Maximum v/c Ratio: 0.61						
Intersection Signal Delay: 6	.6			In	tersectior	1 LOS: A
Intersection Capacity Utiliza	ation 57.3%			IC	ULevel	of Service
Analysis Period (min) 15				.0		
m Volume for 95th percer	ntile queue i	s metere	d hy unstr	eam sign	al	
	nilo quote l		a by upsu	Sum Sign	u1.	
Splits and Phases 3. An	derson Roa	d & Acce	55.2			

Splits and Phases: 3: Anderson Road & Access 2

	<u>∕</u> ≉ _{Ø4}	
37.4 s	22.6 s	
 ↓ Ø6 (R) 		
37.4 s		

2020 with Site Phase I PM

	٦	\mathbf{r}	1	1	Ŧ	-
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ħΜ		ň	*	٠	1
Traffic Volume (vph)	905	60	37	72	101	557
Future Volume (vph)	905	60	37	72	101	557
Lane Util. Factor	0.97	0.95	1.00	1.00	1.00	1.00
Frt	0.991					0.850
Flt Protected	0.955		0.950			
Satd. Flow (prot)	3420	0	1770	1863	1863	1583
Flt Permitted	0.955		0.685			
Satd. Flow (perm)	3420	0	1276	1863	1863	1583
Satd. Flow (RTOR)	14					619
Adi, Flow (vph)	1006	67	41	80	112	619
Lane Group Flow (vph)	1073	0	41	80	112	619
Turn Type	Prot		Perm	NA	NA	Perm
Protected Phases	4			2	6	
Permitted Phases			2	_	Ŭ	6
Total Split (s)	29.0		31.0	31.0	31.0	31.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Act Effct Green (s)	22.9		28.1	28.1	28.1	28.1
Actuated g/C Ratio	0.38		0.47	0.47	0,47	0.47
v/c Ratio	0.82		0.07	0.09	0,13	0.58
Control Delay	22.1		10.1	10.1	4.7	7.9
Queue Delav	0.0		0.0	0.0	0.0	0.0
Total Delay	22.1		10.1	10.1	4.7	7.9
LOS	С		В	В	A	A
Approach Delay	22.1		2	10.1	7.4	
Approach LOS	С			В	A	
Queue Lenath 50th (ft)	162		8	16	8	91
Queue Length 95th (ft)	229		23	37	m15	m172
Internal Link Dist (ft)	215			640	542	
Turn Bay Length (ft)	2.0			2.0	2.2	
Base Capacity (vph)	1404		597	871	871	1070
Starvation Cap Reductn	0		0	0	0	0
Spillback Can Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.76		0.07	0.09	0.13	0.58
	0.70		0.07	0.07	0.10	0.00
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset: 0 (0%), Referenced	to phase 2:1	NBTL an	d 6:SBT, 3	Start of G	reen	
Control Type: Actuated-Co	ordinated					
Maximum v/c Ratio: 0.82						
Intersection Signal Delay: 1	15.8			In	tersection	n LOS: B
Intersection Capacity Utilization	ation 46.2%			IC	U Level	of Service
Analysis Period (min) 15						
m Volume for 95th percer	ntile queue is	s metere	d by upstr	eam sign	al.	
			-	-		
Collite and Dhacaes 2. An	dorcon Doo		cc)			

Splits and Phases: 3: Anderson Road & Access 2

Ø2 (R)	▶ _{Ø4}
31 s	29 s
Ø6 (R)	
31 s	

	۶	-	\mathbf{r}	4	+	•	•	1	1	1	Ļ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	<u></u>	1	ሻሻ	A1⊅		ኘኘ	†	1	۲	eî.	
Traffic Volume (vph)	27	534	1313	400	304	1	735	1	122	1	4	37
Future Volume (vph)	27	534	1313	400	304	1	735	1	122	1	4	37
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	0.95	0.97	1.00	1.00	1.00	1.00	1.00
Frt			0.850						0.850		0.863	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	3539	1583	3433	3539	0	3433	1863	1583	1770	1608	0
Flt Permitted	0.548			0.950			0.950			0.950		
Satd. Flow (perm)	1021	3539	1583	3433	3539	0	3433	1863	1583	1770	1608	0
Satd. Flow (RTOR)			954						136		41	
Adj. Flow (vph)	30	593	1459	444	338	1	817	1	136	1	4	41
Lane Group Flow (vph)	30	593	1459	444	339	0	817	1	136	1	45	0
Turn Type	pm+pt	NA	Free	Prot	NA		Split	NA	pm+ov	Split	NA	
Protected Phases	7	4		3	8		2	2	3	6	6	
Permitted Phases	4		Free						2			
Total Split (s)	9.5	22.5		17.1	30.1		27.9	27.9	17.1	22.5	22.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	4.5	
Act Effct Green (s)	21.9	16.9	77.2	12.6	30.3		23.4	23.4	36.0	6.3	6.3	
Actuated g/C Ratio	0.28	0.22	1.00	0.16	0.39		0.30	0.30	0.47	0.08	0.08	
v/c Ratio	0.09	0.77	0.92	0.79	0.24		0.78	0.00	0.17	0.01	0.27	
Control Delay	14.2	36.0	12.1	43.6	17.6		31.6	20.0	1.9	33.0	17.1	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	14.2	36.0	12.1	43.6	17.6		31.6	20.0	1.9	33.0	17.1	
LOS	В	D	В	D	В		С	В	А	С	В	
Approach Delay		18.9			32.3			27.4			17.4	
Approach LOS		В			С			С			В	
Queue Length 50th (ft)	8	139	0	107	50		185	0	0	0	2	
Queue Length 95th (ft)	23	204	#116	#185	101		#271	4	16	5	32	
Internal Link Dist (ft)		608			332			694			269	
Turn Bay Length (ft)												
Base Capacity (vph)	337	825	1583	560	1388		1041	564	811	412	406	
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Reduced v/c Ratio	0.09	0.72	0.92	0.79	0.24		0.78	0.00	0.17	0.00	0.11	
Intersection Summary												
Cycle Length: 90												
Actuated Cycle Length: 77.2												
Control Type: Actuated-Unco	oordinated											
Maximum v/c Ratio: 0.92												
Intersection Signal Delay: 23	3.7			In	tersectior	n LOS: C						
Intersection Capacity Utilizat	tion 65.1%			IC	CU Level of	of Service	e C					
Analysis Period (min) 15												
# 95th percentile volume e	xceeds ca	oacity, qu	ieue may	be longer	r.							
Queue shown is maximu	m after two	cycles.										

Splits and Phases: 1: Anderson Road & Maricopa-Casa Grande Highway

2025 with Site Phases I & II AM

↓ _{Ø2}	Ø6	€€ Ø3	<u>→</u> _{Ø4}
27.9 s	22.5 s	17.1 s	22.5 s
		▶ _{Ø7} ← _{Ø8}	
		9.5 s 30.1 s	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	**	1	ሻሻ	≜ 1≽		ሻሻ	•	1	5	î,	
Traffic Volume (vph)	24	400	1328	309	569	1	1687	1	493	1	1	20
Future Volume (vph)	24	400	1328	309	569	1	1687	1	493	1	1	20
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	0.95	0.97	1.00	1.00	1.00	1.00	1.00
Frt			0.850						0.850		0.857	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	3539	1583	3433	3539	0	3433	1863	1583	1770	1596	0
Flt Permitted	0.412			0.950			0.950			0.950		
Satd. Flow (perm)	767	3539	1583	3433	3539	0	3433	1863	1583	1770	1596	0
Satd. Flow (RTOR)			973						548		19	
Adj. Flow (vph)	27	444	1476	343	632	1	1874	1	548	1	1	22
Lane Group Flow (vph)	27	444	1476	343	633	0	1874	1	548	1	23	0
Turn Type	Perm	NA	Free	Prot	NA		Split	NA	pm+ov	Split	NA	
Protected Phases		4		3	8		2	2	3	6	6	
Permitted Phases	4		Free						2			
Total Split (s)	22.5	22.5		16.0	38.5		58.0	58.0	16.0	28.5	28.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	4.5	
Act Effct Green (s)	17.2	17.2	106.5	11.5	33.3		53.5	53.5	65.0	6.2	6.2	
Actuated g/C Ratio	0.16	0.16	1.00	0.11	0.31		0.50	0.50	0.61	0.06	0.06	
v/c Ratio	0.22	0.77	0.93	0.93	0.57		1.09	0.00	0.46	0.01	0.21	
Control Delay	44.0	53.1	12.9	79.5	33.2		76.5	14.0	1.8	47.0	27.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	44.0	53.1	12.9	79.5	33.2		76.5	14.0	1.8	47.0	27.7	
LOS	D	D	В	E	С		E	В	А	D	С	
Approach Delay		22.5			49.5			59.6			28.5	
Approach LOS		С			D			E			С	
Queue Length 50th (ft)	16	153	0	121	187		~745	0	0	1	3	
Queue Length 95th (ft)	44	214	#105	#213	253		#910	3	23	6	29	
Internal Link Dist (ft)		608			332			694			269	
Turn Bay Length (ft)												
Base Capacity (vph)	129	598	1583	370	1130		1724	936	1179	399	374	
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Reduced v/c Ratio	0.21	0.74	0.93	0.93	0.56		1.09	0.00	0.46	0.00	0.06	
Intersection Summary												
Cycle Length: 125												
Actuated Cycle Length: 106.5												
Control Type: Actuated-Uncod	ordinated											
Maximum v/c Ratio: 1.09												
Intersection Signal Delay: 44.3	2			In	tersectior	1 LOS: D						
Intersection Capacity Utilization	on 86.0%			IC	U Level o	of Service	εE					
Analysis Period (min) 15												
 Volume exceeds capacity 	, queue is	s theoretic	cally infini	te.								
Queue shown is maximum	after two	cycles.	j									
# 95th percentile volume ex	ceeds ca	pacity, qu	ieue may	be longer	.							
Queue shown is maximum	after two	cycles.		Ū								

2025 with Site Phases I & II PM

2025 with Site Phases I & II PM

Splits and Phases:	1: Anderson Road & Maricopa-Casa Grand	de Highway		
√ _{Ø2}		Ø6	€ Ø3	<u>↓</u> _{Ø4}
58 s		28.5 s	16 s	22.5 s
			← Ø8	
			38.5 s	

2025 with Site Phases I & II AM

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ኘኘ	¢Î		ľ	•	1	ľ	∱ β		ľ	<u></u>	1
Traffic Volume (vph)	207	0	8	31	0	316	32	336	8	50	849	819
Future Volume (vph)	207	0	8	31	0	316	32	336	8	50	849	819
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.850				0.850		0.996				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	1583	0	1770	1863	1583	1770	3525	0	1770	3539	1583
Flt Permitted	0.950			0.752			0.212			0.525		
Satd. Flow (perm)	3433	1583	0	1401	1863	1583	395	3525	0	978	3539	1583
Satd. Flow (RTOR)		27				216		4				910
Adj. Flow (vph)	230	0	9	34	0	351	36	373	9	56	943	910
Lane Group Flow (vph)	230	9	0	34	0	351	36	382	0	56	943	910
Turn Type	Prot	NA		Perm		Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	7	4			8			2			6	7
Permitted Phases				8		8	2			6		6
Total Split (s)	13.0	35.5		22.5	22.5	22.5	24.5	24.5		24.5	24.5	13.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	8.2	24.1		11.4		11.4	26.9	26.9		26.9	26.9	39.6
Actuated g/C Ratio	0.14	0.40		0.19		0.19	0.45	0.45		0.45	0.45	0.66
v/c Ratio	0.49	0.01		0.13		0.74	0.20	0.24		0.13	0.59	0.67
Control Delay	27.8	1.2		18.6		18.3	14.6	9.5		13.3	16.2	3.6
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	27.8	1.2		18.6		18.3	14.6	9.5		13.3	16.2	3.6
LOS	С	А		В		В	В	А		В	В	А
Approach Delay		26.8			18.3			9.9			10.1	
Approach LOS		С			В			А			В	
Queue Length 50th (ft)	40	0		10		44	9	53		11	127	0
Queue Length 95th (ft)	70	2		26		105	38	84		37	#232	44
Internal Link Dist (ft)		256			219			542			694	
Turn Bay Length (ft)												
Base Capacity (vph)	486	830		420		626	177	1584		439	1589	1358
Starvation Cap Reductn	0	0		0		0	0	0		0	0	0
Spillback Cap Reductn	0	0		0		0	0	0		0	0	0
Storage Cap Reductn	0	0		0		0	0	0		0	0	0
Reduced v/c Ratio	0.47	0.01		0.08		0.56	0.20	0.24		0.13	0.59	0.67
Intersection Summary												
Cycle Length: 60												
Actuated Cycle Length: 60												
Offset: 0 (0%), Referenced t	to phase 2:	NBTL and	d 6:SBTL	, Start of	Green							
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.74												
Intersection Signal Delay: 12	2.5			In	itersection	n LOS: B						
Intersection Capacity Utiliza	tion 62.4%			IC	CU Level	of Service	в					
Analysis Period (min) 15												
# 95th percentile volume e	exceeds ca	pacity, qu	eue may	be longe	r.							
Queue shown is maximu	m after two	cycles.	,	0								

2025 with Site Phases I & II AM

Splits and Phases: 2: Anderson Road & Access 1

Ø2 (R)	→ Ø4	
24.5 s	35.5 s	
Ø6 (R)	₽ ^{Ø2}	- ∲ Ø8
24.5 s	13 s	22.5 s

2025 with Site Phases I & II PM

	≯	→	\mathbf{r}	4	+	•	•	1	1	1	ţ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	4		۲	•	1	ň	A		۲	^	7
Traffic Volume (vph)	1006	0	40	17	0	173	24	1003	31	188	831	619
Future Volume (vph)	1006	0	40	17	0	173	24	1003	31	188	831	619
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.95	1.00
Frt		0.850				0.850		0.996				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	1583	0	1770	1863	1583	1770	3525	0	1770	3539	1583
Flt Permitted	0.950			0.769			0.309			0.070		
Satd. Flow (perm)	3433	1583	0	1432	1863	1583	576	3525	0	130	3539	1583
Satd. Flow (RTOR)		71				47		2				688
Adj. Flow (vph)	1118	0	44	19	0	192	27	1114	34	209	923	688
Lane Group Flow (vph)	1118	44	0	19	0	192	27	1148	0	209	923	688
Turn Type	Prot	NA		Perm		pm+ov	Perm	NA		pm+pt	NA	pm+ov
Protected Phases	7	4			8	1		2		1	6	7
Permitted Phases				8		8	2			6		6
Total Split (s)	50.0	72.5		22.5	22.5	17.6	49.9	49.9		17.6	67.5	50.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	45.5	53.4		7.6		26.8	54.2	54.2		77.6	77.6	129.4
Actuated g/C Ratio	0.32	0.38		0.05		0.19	0.39	0.39		0.55	0.55	0.92
v/c Ratio	1.00	0.07		0.25		0.56	0.12	0.84		0.72	0.47	0.45
Control Delay	74.5	1.9		70.4		43.2	34.1	46.8		46.8	20.7	1.1
Queue Delay	0.0	0.0		0.0		0.0	0.0	1.3		0.0	0.0	0.4
Total Delay	74.5	1.9		70.4		43.2	34.1	48.1		46.8	20.7	1.5
LOS	E	А		E		D	С	D		D	С	А
Approach Delay		71.7			45.6			47.8			16.5	
Approach LOS		E			D			D			В	
Queue Length 50th (ft)	~527	0		17		115	17	526		133	275	0
Queue Length 95th (ft)	#680	9		45		188	44	#713		226	348	13
Internal Link Dist (ft)		256			298			542			694	
Turn Bay Length (ft)												
Base Capacity (vph)	1115	805		184		340	223	1366		292	1960	1515
Starvation Cap Reductn	0	0		0		0	0	84		0	0	367
Spillback Cap Reductn	0	0		0		0	0	0		0	0	0
Storage Cap Reductn	0	0		0		0	0	0		0	0	0
Reduced v/c Ratio	1.00	0.05		0.10		0.56	0.12	0.90		0.72	0.47	0.60
Intersection Summary												
Cycle Length: 140												
Actuated Cycle Length: 140												
Offset: 0 (0%), Referenced to	phase 2:	NBTL and	d 6:SBTL	, Start of	Green							
Control Type: Actuated-Coord	linated											
Maximum v/c Ratio: 1.00												
Intersection Signal Delay: 41.0	0			In	tersectio	n LOS: D						
Intersection Capacity Utilization	on 85.7%			IC	U Level	of Service	ε					
Analysis Period (min) 15												
~ Volume exceeds capacity	, queue is	s theoretic	ally infini	ite.								
Queue shown is maximum	after two	o cycles.										
# 95th percentile volume ex	ceeds ca	pacity, qu	eue may	be longer	r.							
Queue shown is maximum	after two	cycles.										

Synchro 9 Report Lanes, Volumes, Timings

2025 with Site Phases I & II PM

Splits and Phases:	2: Anderson Road & Access 1		
V _{Ø1}	Ø2 (R)	→ Ø4	
17.6 s 49	9.9 s	72.5 s	
🖗 Ø6 (R) 📮		\$ ¹ / ₀₇	₹ø8
67.5 s		50 s	22.5 s

	≯	\mathbf{r}	1	1	Ŧ	-
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	×w.		5	*	*	1
Traffic Volume (vph)	186	12	49	190	152	737
Future Volume (vph)	186	12	49	190	152	737
Lane Util. Factor	0.97	0.95	1.00	1.00	1.00	1.00
Frt	0.991					0.850
Flt Protected	0.955		0.950			
Satd. Flow (prot)	3420	0	1770	1863	1863	1583
Flt Permitted	0.955		0.650			
Satd. Flow (perm)	3420	0	1211	1863	1863	1583
Satd. Flow (RTOR)	11					819
Adj. Flow (vph)	207	13	54	211	169	819
Lane Group Flow (vph)	220	0	54	211	169	819
Turn Type	Prot		Perm	NA	NA	Perm
Protected Phases	4			2	6	
Permitted Phases			2			6
Total Split (s)	22.6		37.4	37.4	37.4	37.4
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Act Effct Green (s)	9.0		42.0	42.0	42.0	42.0
Actuated g/C Ratio	0.15		0.70	0.70	0.70	0.70
v/c Ratio	0.42		0.06	0.16	0.13	0.61
Control Delay	24.0		3.5	3.7	0.3	8.7
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	24.0		3.5	3.7	0.3	8.7
LOS	С		А	А	А	А
Approach Delay	24.0			3.7	7.3	
Approach LOS	С			А	А	
Queue Length 50th (ft)	36		5	20	0	163
Queue Length 95th (ft)	60		15	44	m1	207
Internal Link Dist (ft)	215			640	542	
Turn Bay Length (ft)						
Base Capacity (vph)	1039		847	1304	1304	1353
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.21		0.06	0.16	0.13	0.61
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset: 0 (0%), Referenced	to phase 2:I	NBTL an	d 6:SBT,	Start of G	reen	
Control Type: Actuated-Coc	ordinated					
Maximum v/c Ratio: 0.61						
Intersection Signal Delay: 9	.1			In	tersection	n LOS: A
Intersection Capacity Utiliza	ition 57.3%			IC	U Level	of Service
Analysis Period (min) 15						
m Volume for 95th percent	itile queue is	s metere	d by upstr	ream sign	al.	

Splits and Phases: 3: Anderson Road & Access 2

Ø2 (R)	<u>∕</u> ≉ _{Ø4}	
37.4 s	22.6 s	
Ø6 (R)		
37.4 s		

	٦	$\mathbf{\hat{v}}$	1	1	Ŧ	-
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	ΥM		ň	4	•	1
Traffic Volume (vph)	905	60	37	154	332	557
Future Volume (vph)	905	60	37	154	332	557
Lane Util. Factor	0.97	0.95	1.00	1.00	1.00	1.00
Frt	0.991					0.850
Flt Protected	0.955		0.950			
Satd. Flow (prot)	3420	0	1770	1863	1863	1583
Flt Permitted	0.955		0.470			
Satd. Flow (perm)	3420	0	875	1863	1863	1583
Satd. Flow (RTOR)	14					619
Adj. Flow (vph)	1006	67	41	171	369	619
Lane Group Flow (vph)	1073	0	41	171	369	619
Turn Type	Prot		Perm	NA	NA	Perm
Protected Phases	4			2	6	
Permitted Phases			2	_	-	6
Total Split (s)	29.0		31.0	31.0	31.0	31.0
Total Lost Time (s)	4.5		4.5	4.5	4.5	4.5
Act Effct Green (s)	22.9		28.1	28.1	28.1	28.1
Actuated g/C Ratio	0.38		0.47	0.47	0.47	0.47
v/c Ratio	0.82		0.10	0.20	0.42	0.58
Control Delav	22.1		10.6	10.8	13.1	3.6
Oueue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	22.1		10.6	10.8	13.1	3.6
LOS	C		B	B	B	A
Approach Delay	22.1		- U	10.7	7.2	
Approach LOS	C			B	A	
Oueue Length 50th (ff)	162		8	36	88	0
Queue Length 95th (ft)	229		24	70	150	50
Internal Link Dist (ft)	227		21	640	542	00
Turn Bay Length (ft)	210			010	012	
Base Capacity (vph)	1404		409	871	871	1070
Starvation Can Reductn	0		0	0	0	0
Spillback Can Reductin	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.76		0.10	0.20	0.42	0.58
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset 0 (0%) Referenced	to nhaso 2.1	IRTI an	d 6.SBT	Start of G	roon	
Control Type: Actuated Co	a to pridse 2.1	VDIL all	u 0.301,			
Maximum v/c Datio: 0.92	orumateu					
Intersection Signal Delay	1/6			In	torsaction	
Intersection Capacity Hilliz	14.0 ation 60.60/					i LUS. D
Analysis Daried (min) 15	.auun 00.0%			IC	O Level (
Analysis Period (min) 15						

Splits and Phases: 3: Anderson Road & Access 2

<1 Ø2 (R)	▶ _{Ø4}
31 s	29 s
 ✓ Ø6 (R) 	
31 s	

Intersection						
Int Delay, s/veh	4.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	٦	1	4		<u>ک</u>	•
Traffic Vol, veh/h	63	82	157	16	55	108
Future Vol, veh/h	63	82	157	16	55	108
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	0	-
Veh in Median Storage	, # 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	70	91	174	18	61	120

Major/Minor	Minor1	Ν	/lajor1	Ν	/lajor2		
Conflicting Flow All	425	183	0	0	192	0	
Stage 1	183	-	-	-	-	-	
Stage 2	242	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	-	2.218	-	
Pot Cap-1 Maneuver	586	859	-	-	1381	-	
Stage 1	848	-	-	-	-	-	
Stage 2	798	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	560	859	-	-	1381	-	
Mov Cap-2 Maneuver	560	-	-	-	-	-	
Stage 1	848	-	-	-	-	-	
Stage 2	763	-	-	-	-	-	

Approach	WB	NB	SB
HCM Control Delay, s	10.8	0	2.6
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1V	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	560	859	1381	-	
HCM Lane V/C Ratio	-	-	0.125	0.106	0.044	-	
HCM Control Delay (s)	-	-	12.3	9.7	7.7	-	
HCM Lane LOS	-	-	В	А	А	-	
HCM 95th %tile Q(veh)	-	-	0.4	0.4	0.1	-	

4.2					
WBL	WBR	NBT	NBR	SBL	SBT
<u>ک</u>	1	el 👘		<u>ار</u>	•
34	45	146	62	207	184
34	45	146	62	207	184
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	0	-	-	0	-
,# 0	-	0	-	-	0
0	-	0	-	-	0
90	90	90	90	90	90
2	2	2	2	2	2
38	50	162	69	230	204
	4.2 WBL 34 34 34 0 Stop - 0 , # 0 0 90 2 38	4.2 WBL WBR 34 45 34 45 34 45 34 5 0 0 Stop Stop 5top Stop 0 0 , # 0 - 90 90 2 2 38 50	4.2 WBL WBR NBT MBL WBR 146 34 45 146 34 45 146 34 45 146 34 45 146 34 45 146 34 45 146 34 45 146 34 50 500 34 45 146 34 45 146 34 45 146 35 50 60 34 35 36	4.2 NBT NBR WBL WBR NBT NBR MBL If If If 34 146 622 34 455 146 622 34 455 146 622 0 0 0 0 Stop Stop Free Free None - None - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 0 - - 0 - 0 - 0 - 0 - 90 90 90 90 238 50 162 69	4.2 WBL WBR NBT NBR SBL MBL WBR 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 34 45 146 62 207 35 Stop Free Free Free 40 - 0 - 0 41 - 0 - 0 41 - 0 - 0 41 - 0 - 0 42 - 0 - - 42 - 0 - - 40 </td

Major/Minor	Minor1	Ν	/lajor1	М	ajor2		
Conflicting Flow All	861	197	0	0	231	0	
Stage 1	197	-	-	-	-	-	
Stage 2	664	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	- 2	2.218	-	
Pot Cap-1 Maneuver	326	844	-	-	1337	-	
Stage 1	836	-	-	-	-	-	
Stage 2	512	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	270	844	-	-	1337	-	
Mov Cap-2 Maneuver	270	-	-	-	-	-	
Stage 1	836	-	-	-	-	-	
Stage 2	424	-	-	-	-	-	

Approach	WB	NB	SB
HCM Control Delay, s	14.2	0	4.4
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	'BLn1V	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	270	844	1337	-	
HCM Lane V/C Ratio	-	-	0.14	0.059	0.172	-	
HCM Control Delay (s)	-	-	20.5	9.5	8.3	-	
HCM Lane LOS	-	-	С	А	А	-	
HCM 95th %tile Q(veh)	-	-	0.5	0.2	0.6	-	

	≯	-	\mathbf{F}	*	ł	*	•	1	1	1	Ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	۲	* *	1	ሻሻ	4 16		ሻሻ	•	1	ሻ	î,	
Traffic Volume (vph)	37	673	1439	400	383	2	1097	1	125	2	6	50
Future Volume (vph)	37	673	1439	400	383	2	1097	1	125	2	6	50
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	0.95	0.97	1.00	1.00	1.00	1.00	1.00
Frt			0.850		0.999				0.850		0.867	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	3539	1583	3433	3536	0	3433	1863	1583	1770	1615	0
Flt Permitted	0.503			0.950			0.950			0.950		
Satd. Flow (perm)	937	3539	1583	3433	3536	0	3433	1863	1583	1770	1615	0
Satd. Flow (RTOR)			955						139		56	
Adj. Flow (vph)	41	748	1599	444	426	2	1219	1	139	2	7	56
Lane Group Flow (vph)	41	748	1599	444	428	0	1219	1	139	2	63	0
Turn Type	pm+pt	NA	Free	Prot	NA		Split	NA	pm+ov	Split	NA	
Protected Phases	7	4		3	8		2	2	3	6	6	
Permitted Phases	4		Free						2			
Total Split (s)	9.6	30.0		20.2	40.6		47.3	47.3	20.2	22.5	22.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	4.5	
Act Effct Green (s)	30.6	25.5	108.9	15.7	40.0		42.8	42.8	58.5	6.8	6.8	
Actuated g/C Ratio	0.28	0.23	1.00	0.14	0.37		0.39	0.39	0.54	0.06	0.06	
v/c Ratio	0.14	0.90	1.01	0.90	0.33		0.90	0.00	0.15	0.02	0.41	
Control Delay	21.5	56.1	28.5	68.2	26.8		41.9	21.0	1.6	47.5	23.9	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	
Total Delay	21.5	56.1	28.5	68.2	26.8		41.9	21.0	1.6	47.5	23.9	
LOS	С	E	С	E	С		D	С	А	D	С	
Approach Delay		37.1			47.9			37.8			24.6	
Approach LOS		D			D			D			С	
Queue Length 50th (ft)	16	264	~23	157	117		400	0	0	1	5	
Queue Length 95th (ft)	40	#394	#288	#259	169		#567	4	18	9	47	
Internal Link Dist (ft)		608			332			694			269	
Turn Bay Length (ft)												
Base Capacity (vph)	302	829	1583	495	1299		1349	732	914	292	314	
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Reduced v/c Ratio	0.14	0.90	1.01	0.90	0.33		0.90	0.00	0.15	0.01	0.20	
Intersection Summary												
Cycle Length: 120												
Actuated Cycle Length: 108.	9											
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 1.01												
Intersection Signal Delay: 39	.1			In	itersection	ו LOS: D						
Intersection Capacity Utilizat	Intersection Capacity Utilization 79.2% ICU Level of Service D											
Analysis Period (min) 15												
 Volume exceeds capacity Ouque shown is maximum 	y, queue is	s theoretic	cally infini	ite.								
# Q5th porcoptilo volume or		nacity cu		ho longo	r							
 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles. 												

Splits and Phases:	1: Anderson Road & Maricopa-Ca	sa Grande Highway				
√ _{Ø2}		▶ _{Ø6}	€ Ø3		<u></u>	
47.3 s	2	22.5 s	20.2 s		30 s	
			≯ _{Ø7}	← Ø8		
			9.6 s	40.6 s		

	≯	+	\mathbf{r}	4	Ļ	•	•	1	*	1	ŧ	~
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	5	* *	1	ሻሻ	≜1 5		ሻሻ	•	1	5	1.	
Traffic Volume (vph)	33	504	1728	310	717	2	1923	1	494	2	2	27
Future Volume (vph)	33	504	1728	310	717	2	1923	1	494	2	2	27
Lane Util. Factor	1.00	0.95	1.00	0.97	0.95	0.95	0.97	1.00	1.00	1.00	1.00	1.00
Frt			0.850						0.850		0.859	
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	3539	1583	3433	3539	0	3433	1863	1583	1770	1600	0
Flt Permitted	0.219			0.950			0.950			0.950		
Satd. Flow (perm)	408	3539	1583	3433	3539	0	3433	1863	1583	1770	1600	0
Satd. Flow (RTOR)			977						489		8	
Adj. Flow (vph)	37	560	1920	344	797	2	2137	1	549	2	2	30
Lane Group Flow (vph)	37	560	1920	344	799	0	2137	1	549	2	32	0
Turn Type	Perm	NA	Free	Prot	NA		Split	NA	pm+ov	Split	NA	
Protected Phases		4		3	8		2	2	3	6	6	
Permitted Phases	4		Free						2			
Total Split (s)	26.0	26.0		18.0	44.0		83.5	83.5	18.0	22.5	22.5	
Total Lost Time (s)	4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	4.5	
Act Effct Green (s)	21.5	21.5	139.7	13.5	39.5		79.0	79.0	92.5	7.7	7.7	
Actuated g/C Ratio	0.15	0.15	1.00	0.10	0.28		0.57	0.57	0.66	0.06	0.06	
v/c Ratio	0.60	1.03	1.21	1.04	0.80		1.10	0.00	0.45	0.02	0.34	
Control Delay	93.2	103.2	110.5	119.7	53.7		84.0	14.0	2.0	62.0	60.3	
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.3	0.0	0.1	0.0	0.0	
Total Delay	93.2	103.2	110.5	119.7	53.7		84.3	14.0	2.1	62.0	60.3	
LOS	F	F	F	F	D		F	В	А	E	E	
Approach Delay		108.7			73.6			67.5			60.4	
Approach LOS		F			E			E			E	
Queue Length 50th (ft)	32	~284	~634	~172	354		~1130	0	11	2	21	
Queue Length 95th (ft)	#91	#419	#908	#282	448		#1307	3	35	12	57	
Internal Link Dist (ft)		608			332			694			269	
Turn Bay Length (ft)			1 = 0.0									
Base Capacity (vph)	62	544	1583	331	1000		1941	1053	1213	228	213	
Starvation Cap Reductn	0	0	0	0	0		190	0	123	0	0	
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	
Reduced V/c Ratio	0.60	1.03	1.21	1.04	0.80		1.22	0.00	0.50	0.01	0.15	
Intersection Summary												
Cycle Length: 150	-											
Actuated Cycle Length: 139.	/											
Control Type: Actuated-Unco	ordinated											
Maximum v/c Ratio: 1.21	0					100 5						
Intersection Signal Delay: 84	.8			In	itersection	1 LOS: F	F					
Intersection Capacity Utilizati	ion 96.8%			IC	U Level o	of Service	9 F					
Analysis Period (min) 15												
 volume exceeds capacity 	y, queue is	s theoretic	cally infini	ie.								
Queue shown is maximun	n arter two) cycles.		halens	_							
# 95th percentile volume ex	xceeds ca	pacity, qu	ieue may	be longe	ſ.							
Queue snown is maximun	n aner two) cycles.										

Splits and Phases:	1: Anderson Road & Maricopa-Casa Grande Highway			
√ ø₂		Ø6	€ Ø3	A ₀₄
83.5 s		22.5 s	18 s	26 s
			← Ø8	
			44 s	

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	ţ,		5	•	1	۲.	<u> ተተ</u> ኑ		ሻ	***	1
Traffic Volume (vph)	207	0	8	31	0	372	32	644	8	79	948	819
Future Volume (vph)	207	0	8	31	0	372	32	644	8	79	948	819
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	1.00
Frt		0.850				0.850		0.998				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	1583	0	1770	1863	1583	1770	5075	0	1770	5085	1583
Flt Permitted	0.950			0.752			0.189			0.326		
Satd. Flow (perm)	3433	1583	0	1401	1863	1583	352	5075	0	607	5085	1583
Satd. Flow (RTOR)		27				109		3				910
Adj. Flow (vph)	230	0	9	34	0	413	36	716	9	88	1053	910
Lane Group Flow (vph)	230	9	0	34	0	413	36	725	0	88	1053	910
Turn Type	Prot	NA		Perm		Perm	Perm	NA		Perm	NA	pm+ov
Protected Phases	7	4			8			2			6	7
Permitted Phases				8		8	2			6		6
Total Split (s)	15.0	37.5		22.5	22.5	22.5	22.5	22.5		22.5	22.5	15.0
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	9.5	29.8		15.8		15.8	21.2	21.2		21.2	21.2	35.2
Actuated g/C Ratio	0.16	0.50		0.26		0.26	0.35	0.35		0.35	0.35	0.59
v/c Ratio	0.42	0.01		0.09		0.83	0.29	0.40		0.41	0.59	0.70
Control Delay	25.0	1.1		16.1		31.1	23.1	14.8		24.3	18.4	4.0
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	25.0	1.1		16.1		31.1	23.1	14.8		24.3	18.4	4.0
LOS	С	А		В		С	С	В		С	В	A
Approach Delay		24.1			30.0			15.2			12.3	
Approach LOS		С			С			В			В	
Queue Length 50th (ft)	38	0		9		97	8	58		26	121	0
Queue Length 95th (ft)	67	2		26		#226	m25	105		#70	163	44
Internal Link Dist (ft)		256			219			542			694	
Turn Bay Length (ft)												
Base Capacity (vph)	600	882		420		551	124	1798		215	1799	1316
Starvation Cap Reductn	0	0		0		0	0	0		0	0	0
Spillback Cap Reductn	0	0		0		0	0	0		0	0	0
Storage Cap Reductn	0	0		0		0	0	0		0	0	0
Reduced v/c Ratio	0.38	0.01		0.08		0.75	0.29	0.40		0.41	0.59	0.69
Intersection Summary												
Cycle Length: 60												
Actuated Cycle Length: 60												
Offset: 0 (0%), Referenced to	phase 2:	NBTL and	d 6:SBTL	, Start of	Green							
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 0.83												
Intersection Signal Delay: 16.	.0			In	tersection	ו LOS: B						
Intersection Capacity Utilizati	on 62.4%			IC	U Level	of Service	в					
Analysis Period (min) 15												
# 95th percentile volume ex	kceeds ca	pacity, qu	eue may	be longe	r.							
Queue shown is maximum	n after two	cycles.										
m Volume for 95th percenti	ile queue i	is metered	d by upst	ream sign	ial.							

Splits and Phases: 2: Anderson Road & Access 1

Ø2 (R)	→ Ø4	
22.5 s	37.5 s	
Ø6 (R)	₽ [™] _{Ø7}	
22.5 s	15 s	22.5 s

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻሻ	î,		5	•	1	5	#†\$		5	***	1
Traffic Volume (vph)	1006	0	40	17	0	209	24	1204	31	281	1141	619
Future Volume (vph)	1006	0	40	17	0	209	24	1204	31	281	1141	619
Lane Util. Factor	0.97	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	1.00	0.91	1.00
Frt		0.850				0.850		0.996				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	3433	1583	0	1770	1863	1583	1770	5065	0	1770	5085	1583
Flt Permitted	0.950			0.769			0.207			0.082		
Satd. Flow (perm)	3433	1583	0	1432	1863	1583	386	5065	0	153	5085	1583
Satd. Flow (RTOR)		47				47		3				688
Adj. Flow (vph)	1118	0	44	19	0	232	27	1338	34	312	1268	688
Lane Group Flow (vph)	1118	44	0	19	0	232	27	1372	0	312	1268	688
Turn Type	Prot	NA		Perm		pm+ov	Perm	NA		pm+pt	NA	pm+ov
Protected Phases	7	4			8	1		2		1	6	. 7
Permitted Phases				8		8	2			6		6
Total Split (s)	49.4	71.9		22.5	22.5	24.8	43.3	43.3		24.8	68.1	49.4
Total Lost Time (s)	4.5	4.5		4.5	4.5	4.5	4.5	4.5		4.5	4.5	4.5
Act Effct Green (s)	44.9	52.8		7.6		35.7	45.9	45.9		78.2	78.2	129.4
Actuated g/C Ratio	0.32	0.38		0.05		0.26	0.33	0.33		0.56	0.56	0.92
v/c Ratio	1.02	0.07		0.25		0.53	0.21	0.83		0.77	0.45	0.45
Control Delay	77.8	6.2		70.4		38.7	44.0	49.2		49.2	19.6	1.1
Queue Delay	0.0	0.0		0.0		0.0	0.0	0.0		0.0	0.0	0.4
Total Delay	77.8	6.2		70.4		38.7	44.0	49.2		49.2	19.6	1.5
LOS	E	А		E		D	D	D		D	В	А
Approach Delay		75.0			41.1			49.1			18.2	
Approach LOS		E			D			D			В	
Queue Length 50th (ft)	~552	0		17		137	19	447		217	256	0
Queue Length 95th (ft)	#688	22		45		217	50	#561		#354	310	13
Internal Link Dist (ft)		256			298			542			694	
Turn Bay Length (ft)												
Base Capacity (vph)	1101	786		184		438	126	1663		405	2839	1515
Starvation Cap Reductn	0	0		0		0	0	0		0	0	367
Spillback Cap Reductn	0	0		0		0	0	0		0	0	0
Storage Cap Reductn	0	0		0		0	0	0		0	0	0
Reduced v/c Ratio	1.02	0.06		0.10		0.53	0.21	0.83		0.77	0.45	0.60
Intersection Summary												
Cycle Length: 140												
Actuated Cycle Length: 140												
Offset: 0 (0%), Referenced to	phase 2:	NBTL and	6:SBTL	, Start of (Green							
Control Type: Actuated-Coord	dinated											
Maximum v/c Ratio: 1.02												
Intersection Signal Delay: 40.8	8			In	tersectio	n LOS: D						
Intersection Capacity Utilization	on 86.1%			IC	U Level	of Service	εE					
Analysis Period (min) 15												
~ Volume exceeds capacity	, queue is	s theoretic	ally infini	te.								
Queue shown is maximum	after two	o cycles.										
# 95th percentile volume ex	ceeds ca	pacity, qu	eue may	be longer	·.							
Queue shown is maximum	after two	o cycles.										

Splits and Phases:	2: Anderson Road & Access 1		
S _{Ø1}	• 1 Ø2 (R)	→ _{Ø4}	
24.8 s	43.3 s	71.9 s	
Ø6 (R)		₽ Ø7	∲ Ø8
68.1 s		49.4 s	22.5 s

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	۲W		5	*	•	1
Traffic Volume (vph)	186	12	49	498	251	737
Future Volume (vph)	186	12	49	498	251	737
Lane Util. Factor	0.97	0.95	1.00	1.00	1.00	1.00
Frt	0.991					0.850
Flt Protected	0.955		0.950			
Satd. Flow (prot)	3420	0	1770	1863	1863	1583
Flt Permitted	0.955	Ű	0.588		1000	1000
Satd Flow (perm)	3420	0	1095	1863	1863	1583
Satd Flow (RTOR)	12	Ŭ	1070	1000	1000	1000
Adi Flow (vph)	207	13	54	553	279	819
Lane Group Flow (vph)	207	0	54	553	279	819
Turn Tyne	Prot	0	Perm	NΔ	NΔ	nm±ov
Protected Phases	1		T GHH	2	6	μπ±0ν Λ
Parmittad Dhasas	4		C	Z	0	4
r cittilleu Fildses Total Split (c)	2E 0		25.0	2E 0	25.0	0 25 0
Total Last Time (a)	20.U		30.U	30.U	30.0	20.U
TUIDI LUST TIME (S)	4.0		4.5	4.5	4.0	4.5
Activities and all Dette	12.9		38.1	38.1	38.1	00.0
Actuated g/C Ratio	0.22		0.64	0.64	0.64	1.00
V/C Ratio	0.29		0.08	0.47	0.24	0.52
Control Delay	18.4		6.1	8.5	0.6	9.2
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	18.4		6.1	8.5	0.6	9.2
LOS	В		А	А	А	А
Approach Delay	18.4			8.3	7.0	
Approach LOS	В			А	А	
Queue Length 50th (ft)	32		6	84	0	209
Queue Length 95th (ft)	48		24	207	5	258
Internal Link Dist (ft)	215			640	542	
Turn Bay Length (ft)						
Base Capacity (vph)	1176		694	1181	1181	1563
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.19		0.08	0.47	0.24	0.52
Intersection Summarv						
Cycle Length: 60						
Actuated Cycle Length: 60)					
Offset: 0 (0%) Reference	d to nhaco 2.1	VIRTI an	d 6.CRT	Start of C	reen	
Control Type: Actuated C	ordinated	VDIL all	u 0.301,			
Maximum v/c Datio: 0.52	oorunateu					
Intersection Signal Delay	07			In	torcostio	
Intersection Signal Delay:	0.1					of Comiler
Analysis Dariad (min) 15	2011011 37.3%			IC	U Level	UI SEIVICE
Analysis Period (min) 15						

Splits and Phases: 3: Anderson Road & Access 2



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	N M		5	٠	*	1
Traffic Volume (vph)	905	60	37	355	641	557
Future Volume (vph)	905	60	37	355	641	557
Lane Util Factor	0.97	0.95	1 00	1 00	1 00	1 00
Frt	0 991	0.70	1.00	1.00	1.00	0.850
Flt Protected	0.955		0 950			0.000
Satd Flow (prot)	3420	0	1770	1863	1863	1583
Elt Permitted	0.955	U	0 185	1005	1005	1000
Satd Flow (perm)	3420	0	345	1863	1863	1583
Sate Flow (PTOR)	12	U	545	1005	1005	1000
Adi Flow (vph)	1006	67	/1	30/	712	610
Lano Group Flow (vph)	1000	07	41	20/	712	610
	Drot	0	Dorm	574 ΝΛ		
Protoctod Phases			FCIIII	N/A ک	NA 4	μπ+0v 1
Protected Plidses	4		n	2	0	4
Tetal Calit (a)	2/ /		2	22.4	11	0
Total Split (S)	26.6		33.4	33.4	33.4	26.6
I OLAI LOST I IME (S)	4.5		4.5	4.5	4.5	4.5
Act Effect Green (s)	21.7		29.3	29.3	29.3	60.0
Actuated g/C Ratio	0.36		0.49	0.49	0.49	1.00
v/c Ratio	0.86		0.24	0.43	0.78	0.39
Control Delay	26.6		13.9	12.0	21.0	0.7
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	26.6		13.9	12.0	21.0	0.7
LOS	С		В	В	С	А
Approach Delay	26.6			12.2	11.6	
Approach LOS	С			В	В	
Queue Length 50th (ft)	176		8	87	202	0
Queue Length 95th (ft)	#282		29	148	#388	0
Internal Link Dist (ft)	215			640	542	
Turn Bay Length (ft)						
Base Capacity (vph)	1267		168	909	909	1570
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.85		0.24	0.43	0.78	0.39
Intersection Summary						
Cycle Length: 60						
Actuated Cycle Length: 60						
Offset: 0 (0%), Referenced	to phase 2:I	NBTL an	d 6:SBT, 3	Start of G	reen	
Control Type: Actuated-Coc	ordinated					
Maximum v/c Ratio: 0.86						
Intersection Signal Delay: 1	7.4			In	tersectio	n LOS: B
Intersection Capacity Utiliza	ation 68.9%			IC	U Level	of Service
Analysis Period (min) 15				.0	5 20101	2. 201100
# 95th percentile volume	exceeds car	nacity or	IELIE may	he longer		
	im after two	cvcles	iouo may	Se longel	•	
		cycics.				

Splits and Phases:	3: Anderson Road & Access 2		
Ø2 (R)		** _{Ø4}	
33.4 s		26.6 s	
Ø6 (R)			
33.4 s			

Intersection						
Int Delay, s/veh	5.7					
-						
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>۲</u>	1	4		- ሽ	↑
Traffic Vol, veh/h	91	210	337	26	114	149
Future Vol, veh/h	91	210	337	26	114	149
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	0	-
Veh in Median Storage,	# 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	101	233	374	29	127	166
		_00		- '		

Major/Minor	Minor1	Ν	/lajor1	Ν	lajor2				
Conflicting Flow All	808	389	0	0	403	0			
Stage 1	389	-	-	-	-	-			
Stage 2	419	-	-	-	-	-			
Critical Hdwy	6.42	6.22	-	-	4.12	-			
Critical Hdwy Stg 1	5.42	-	-	-	-	-			
Critical Hdwy Stg 2	5.42	-	-	-	-	-			
Follow-up Hdwy	3.518	3.318	-	-	2.218	-			
Pot Cap-1 Maneuver	350	659	-	-	1156	-			
Stage 1	685	-	-	-	-	-			
Stage 2	664	-	-	-	-	-			
Platoon blocked, %			-	-		-			
Mov Cap-1 Maneuver	312	659	-	-	1156	-			
Mov Cap-2 Maneuver	430	-	-	-	-	-			
Stage 1	685	-	-	-	-	-			
Stage 2	591	-	-	-	-	-			

Approach	WB	NB	SB
HCM Control Delay, s	14.2	0	3.7
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1\	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	430	659	1156	-	
HCM Lane V/C Ratio	-	-	0.235	0.354	0.11	-	
HCM Control Delay (s)	-	-	15.9	13.4	8.5	-	
HCM Lane LOS	-	-	С	В	А	-	
HCM 95th %tile Q(veh)	-	-	0.9	1.6	0.4	-	

Intersection						
Int Delay, s/veh	6.3					
						~~~
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>۲</u>	1	- î>		- ሽ	<b>↑</b>
Traffic Vol, veh/h	52	128	264	93	398	303
Future Vol, veh/h	52	128	264	93	398	303
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	0	-
Veh in Median Storage,	# 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	58	142	293	103	442	337
	00		270	.00		207

Major/Minor	Minor1	Ν	/lajor1	N	Najor2				
Conflicting Flow All	1566	345	0	0	397	0			
Stage 1	345	-	-	-	-	-			
Stage 2	1221	-	-	-	-	-			
Critical Hdwy	6.42	6.22	-	-	4.12	-			
Critical Hdwy Stg 1	5.42	-	-	-	-	-			
Critical Hdwy Stg 2	5.42	-	-	-	-	-			
Follow-up Hdwy	3.518	3.318	-	-	2.218	-			
Pot Cap-1 Maneuver	122	698	-	-	1162	-			
Stage 1	717	-	-	-	-	-			
Stage 2	279	-	-	-	-	-			
Platoon blocked, %			-	-		-			
Mov Cap-1 Maneuver	76	698	-	-	1162	-			
Mov Cap-2 Maneuver	146	-	-	-	-	-			
Stage 1	717	-	-	-	-	-			
Stage 2	173	-	-	-	-	-			

Approach	WB	NB	SB
HCM Control Delay, s	21.1	0	5.7
HCM LOS	С		

Minor Lane/Major Mvmt	NBT	NBRWBL	1WBLn2	SBL	SBT	
Capacity (veh/h)	-	- 1	6 698	1162	-	
HCM Lane V/C Ratio	-	- 0.3	6 0.204	0.381	-	
HCM Control Delay (s)	-	- 44	.9 11.5	10	-	
HCM Lane LOS	-	-	E B	А	-	
HCM 95th %tile Q(veh)	-	- 1	.7 0.8	1.8	-	
Intersection						
------------------------	------	------	------	------	------	----------
Int Delay, s/veh	4.7					
			NDT	NDD	0.01	0.D.T
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	۰¥		- î>		- ሽ	<b>↑</b>
Traffic Vol, veh/h	55	172	191	18	34	206
Future Vol, veh/h	55	172	191	18	34	206
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	61	191	212	20	38	229

Major/Minor	Minor1	Ν	/lajor1	N	lajor2		
Conflicting Flow All	526	222	0	0	232	0	
Stage 1	222	-	-	-	-	-	
Stage 2	304	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	- 2	2.218	-	
Pot Cap-1 Maneuver	512	818	-	-	1336	-	
Stage 1	815	-	-	-	-	-	
Stage 2	748	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	497	818	-	-	1336	-	
Mov Cap-2 Maneuver	497	-	-	-	-	-	
Stage 1	815	-	-	-	-	-	
Stage 2	727	-	-	-	-	-	

Approach	WB	NB	SB
HCM Control Delay, s	12.9	0	1.1
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRWBLr	1 SBL	SBT	
Capacity (veh/h)	-	- 7(	7 1336	-	
HCM Lane V/C Ratio	-	- 0.35	7 0.028	-	
HCM Control Delay (s)	-	- 12	9 7.8	-	
HCM Lane LOS	-	-	B A	-	
HCM 95th %tile Q(veh)	-	- 1	6 0.1	-	

Intersection						
Int Delay, s/veh	3.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		el 👘		۲.	•
Traffic Vol, veh/h	36	112	245	61	111	245
Future Vol, veh/h	36	112	245	61	111	245
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	0	-
Veh in Median Storage,	# 0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	124	272	68	123	272

Major/Minor	Minor1	Ν	/lajor1	Ν	lajor2		
Conflicting Flow All	825	306	0	0	340	0	
Stage 1	306	-	-	-	-	-	
Stage 2	519	-	-	-	-	-	
Critical Hdwy	6.42	6.22	-	-	4.12	-	
Critical Hdwy Stg 1	5.42	-	-	-	-	-	
Critical Hdwy Stg 2	5.42	-	-	-	-	-	
Follow-up Hdwy	3.518	3.318	-	-	2.218	-	
Pot Cap-1 Maneuver	342	734	-	-	1219	-	
Stage 1	747	-	-	-	-	-	
Stage 2	597	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	307	734	-	-	1219	-	
Mov Cap-2 Maneuver	307	-	-	-	-	-	
Stage 1	747	-	-	-	-	-	
Stage 2	537	-	-	-	-	-	

Approach	WB	NB	SB
HCM Control Delay, s	14.4	0	2.6
HCM LOS	В		

Minor Lane/Major Mvmt	NBT	NBRW	/BLn1	SBL	SBT
Capacity (veh/h)	-	-	548	1219	-
HCM Lane V/C Ratio	-	-	0.3	0.101	-
HCM Control Delay (s)	-	-	14.4	8.3	-
HCM Lane LOS	-	-	В	А	-
HCM 95th %tile Q(veh)	-	-	1.3	0.3	-

Intersection						
Int Delay, s/veh	9.7					
-						
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		୍ କ	- î÷		۰¥	
Traffic Vol, veh/h	53	0	0	40	117	228
Future Vol, veh/h	53	0	0	40	117	228
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,	# -	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	59	0	0	44	130	253
	• •	v	•			_00

Major/Minor	Major1	Maj	or2		Minor2			
Conflicting Flow All	44	0	-	0	140	22		
Stage 1	-	-	-	-	22	-		
Stage 2	-	-	-	-	118	-		
Critical Hdwy	4.12	-	-	-	6.42	6.22		
Critical Hdwy Stg 1	-	-	-	-	5.42	-		
Critical Hdwy Stg 2	-	-	-	-	5.42	-		
Follow-up Hdwy	2.218	-	-	-	3.518	3.318		
Pot Cap-1 Maneuver	1564	-	-	-	853	1055		
Stage 1	-	-	-	-	1001	-		
Stage 2	-	-	-	-	907	-		
Platoon blocked, %		-	-	-				
Mov Cap-1 Maneuver	r 1564	-	-	-	821	1055		
Mov Cap-2 Maneuver	r -	-	-	-	821	-		
Stage 1	-	-	-	-	1001	-		
Stage 2	-	-	-	-	873	-		

Approach	EB	WB	SB
HCM Control Delay, s	7.4	0	11.2
HCM LOS			В

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLn1
Capacity (veh/h)	1564	-	-	- 962
HCM Lane V/C Ratio	0.038	-	-	- 0.398
HCM Control Delay (s)	7.4	0	-	- 11.2
HCM Lane LOS	А	А	-	- B
HCM 95th %tile Q(veh)	0.1	-	-	- 1.9

<u> SBF</u>	SBR
- Y	
) 76	148
) 76	148
0 0	0
e Stop	Stop
- 6	None
- 0	-
- 0	-
- 0	-
) 90	90
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<u> </u>	~
30 30 30 30 30 30 30 30	IR SBL   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 76   30 70   30 90

Major/Minor	Major1	Maj	or2		Minor2			
Conflicting Flow All	144	0	-	0	456	72		
Stage 1	-	-	-	-	72	-		
Stage 2	-	-	-	-	384	-		
Critical Hdwy	4.12	-	-	-	6.42	6.22		
Critical Hdwy Stg 1	-	-	-	-	5.42	-		
Critical Hdwy Stg 2	-	-	-	-	5.42	-		
Follow-up Hdwy	2.218	-	-	-	3.518	3.318		
Pot Cap-1 Maneuver	1438	-	-	-	562	990		
Stage 1	-	-	-	-	951	-		
Stage 2	-	-	-	-	688	-		
Platoon blocked, %		-	-	-				
Mov Cap-1 Maneuver	⁻ 1438	-	-	-	487	990		
Mov Cap-2 Maneuver	· _	-	-	-	487	-		
Stage 1	-	-	-	-	951	-		
Stage 2	-	-	-	-	596	-		

Approach	EB	WB	SB	
HCM Control Delay, s	7.9	0	12.4	
HCM LOS			В	

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR SBLr
Capacity (veh/h)	1438	-	-	- 73
HCM Lane V/C Ratio	0.134	-	-	- 0.3
HCM Control Delay (s)	7.9	0	-	- 12
HCM Lane LOS	А	А	-	-
HCM 95th %tile Q(veh)	0.5	-	-	- 1.